

91 W. Colt Square Dr. Suite 3 / Fayetteville, AR 72703 PH: 479-442-9350 * FAX: 479-521-9350

DRAINAGE DESIGN COMPUTATIONS

For

CENTRAL ELECTRIC

BA No. 15-198

FLORANCE AVENUE TONTITOWN, ARKANSAS

JULY 7, 2015

SUBMITTED TO: CITY OF TONTITOWN

This Drainage Study has been reviewed for general compliance with the City of Tontitown Zoning and Planning Ordinances. Oversight of any regulations does not relieve the Owner of their responsibility to comply with all regulations.

Terry W. Carpenter, P.E. Tontitown City Engineer, July 09, 2015

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PRE AND POST DRAINAGE AREA MAP

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REFERENCES

PROJECT OWNER AND DEVELOPER:

Central Electric Contractors 228 S. 40th Street A Springdale, AR 72756

I don't think this is the correct address.

PROJECT TITLE:

The following information is the drainage report for Central Electric.

PROJECT LOCATION:

This project is located at 284 South Mantegani Road, in Tontitown, Arkansas in Washington County. See the attached vicinity map for the exact location.

PROJECT DESCRIPTION:

This project site is a 1.27 acre tract in the existing Tontitown Plaza subdivision. Improvements to the site will consist of paving parking lot, drives and adding a 5,000 s.f. building. See the large scale development plan for details.

SITE DRAINAGE:

This project is a small part of a large drainage basin that flows into Brush Creek and eventually discharging into the Illinois River. The soil type for the drainage basin found in the Natural Resources Conservation Service Soil Survey is Captina silt loam which belongs to Hydrologic Soil Group C and Johnsburg which belongs to Hydrologic Soil Group D. The majority of the soil types in the basin are in Soil Group C.

Soil group C soils have low infiltration rates when thoroughly wetted and consist chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine to fine texture. These soils have a low rate of water transmission.

Group D soils have very low infiltration rates when thoroughly wetted and consist chiefly of clay soils with a high swelling potential, soils with a permanent high water table, soils with a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. These soils have a very low rate of water transmission (0-0.05 in/hr).

No portion of this property is located within Flood Zone "AE" and the "Floodway" as determined by the National Flood Insurance Program's Flood Insurance Rate Map for Washington County, Arkansas (Map No. 05143C0045 F, May 16, 2008).

Currently, the runoff from this site sheet flows from the northwest to southeast. Once developed, the runoff from the site will continue in the same general pattern.

AREA DRAIANGE PROBLEMS:

To our knowledge, there are no known drainage problems in this area at this time.

DRAINAGE DESIGN:

Runoff Coefficients for each drainage basin was selected from the Tontitown Drainage Manual. The coefficient was selected based on the amount of pervious and impervious area in the basin. A composite runoff curve number was then calculated for the basin. The composite runoff coefficients are as follows:

		(50%)	(50%)			
Cover		Type C	Type D			
Open Space	ce (good cover)	86	89			
Gravel	,	89				
Imperviou	IS	98	98			
Pre Deve	eloped Basin1		(0.28 ac)			
86	Open space		0.28 ac			
Pre Deve	eloped Basin 2		(0.75 ac)			
89	Open space		0.75 ac			
Post Dev	Post Developed Basin 1					
86	Open space		0.08 ac			
98	Impervious		0.15 ac			

90 Post developed composite curve number

Post Dev	veloped Basin 2	(0.80 ac)
89	Open space	0.32 ac
89	Gravel	0.27 ac
98	Impervious	0.21 ac

91 Post developed composite curve number

Basin 1 Runoff Calculations

The 2-year through 100-year frequency storm events for pre- and post-development flows were calculated using the drainage program Hydraflow Hydrographs (SCS Method). The post developed peak flows decreased slightly due to decreasing the basin size.

Basin 1:

The peak runoff will slightly increase in basin 1 due to the increase in impervious area.

(Peak flow in cfs)

Design			
Storm	Pre-Dev.	Post-Dev	Difference
2-yr	0.75	0.69	-0.06
5-yr	0.89	0.80	009
10-yr	1.23	1.08	-0.15
25-yr	1.47	1.28	-0.19
100-yr	1.89	1.62	-0.27

Basin 2:

The peak runoff will slightly increase in basin 2 due to the increase in impervious area. However, detention is proposed to reduce the peak runoff to pre developed conditions.

(1	Peak	flow	in	cfs)
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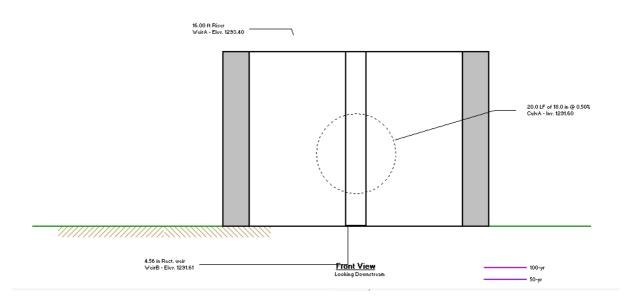
Design Storm	Pre-Dev.	Post-Dev	Difference
2-yr	1.59	1.57	-0.02
5-yr	1.88	1.77	-0.11
10-yr	2.55	2.09	-0.46
25-yr	3.03	2.36	-0.67
100-yr	3.85	3.67	-0.18

DETENTION BASIN 1:

The 100 year WSEL in the pond is 1293.5 and the berm is at elevation 1294.5. This provides 12 inches of freeboard. The release structure consists of a 4'x4' riser with a 4.5" weir and an 18" corrugated metal pipe outlet. See the grading plan for details.

Design Storm	Storage Cu. Ft	Elevation
2-yr	4,895	1287.41
10-yr	8,986	1287.84
25-yr	11,483	1288.04
50-yr	13,885	1288.15
100-yr	15,743	1288.23

Top of pond Elev. 1294.00



EROSION AND SEDIMENT CONTROL:

Erosion and sediment control will be achieved through the use of silt fences and rip rap ditch checks.

CONCLUSION:

The improvements to the site consist of adding parking, drives and a building. The peak runoff will increase due to the improvements replacing pervious area with impervious area. However, detention is proposed to lower the peak runoff to pre developed conditions.

CERTIFICATION:

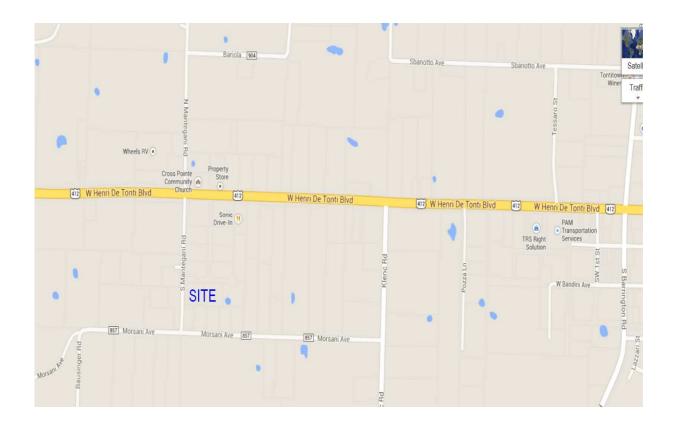
I, Geoffrey H. Bates, Registered Professional Engineer No. 9810 in the State of Arkansas, hereby certify that the drainage studies, reports, calculations, designs, and specifications contained in this report have been prepared in accordance with the requirements of the City of Tontitown. Further, I hereby acknowledge that the review of the drainage studies, reports, calculations, designs, and specifications by the City of Tontitown or its representatives cannot and does not relieve me from any professional responsibility or liability."

Sincerely,

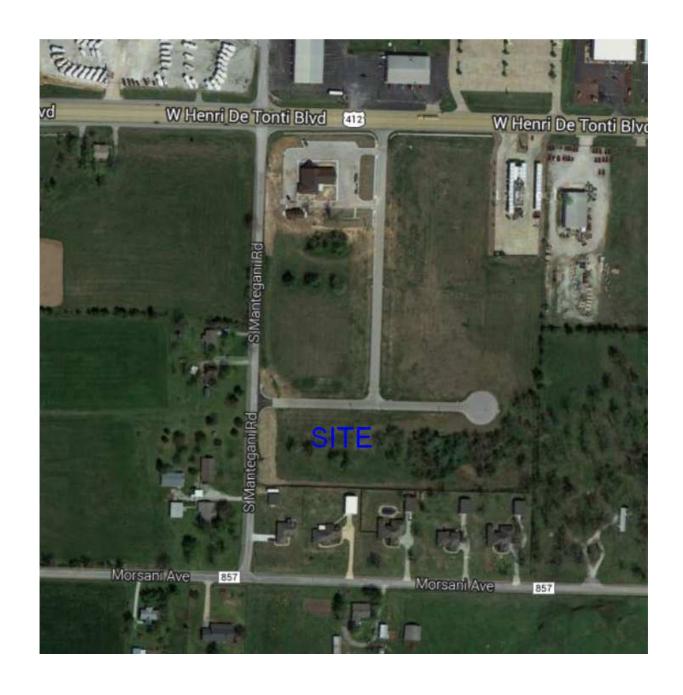
Bates & Associates, Inc.

Deoffrey Bates

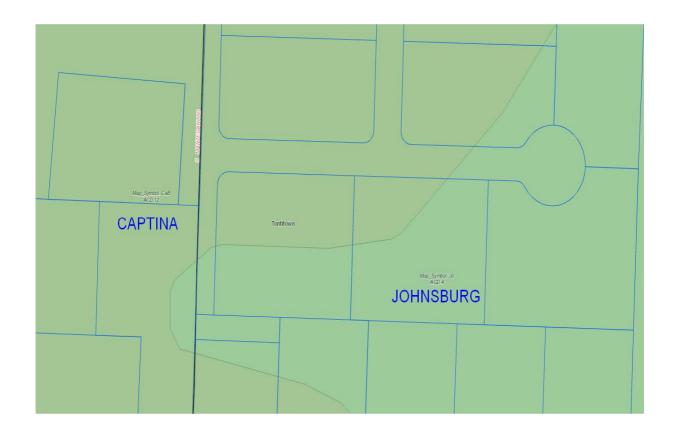
Geoffrey H. Bates, P.E. President of Engineering



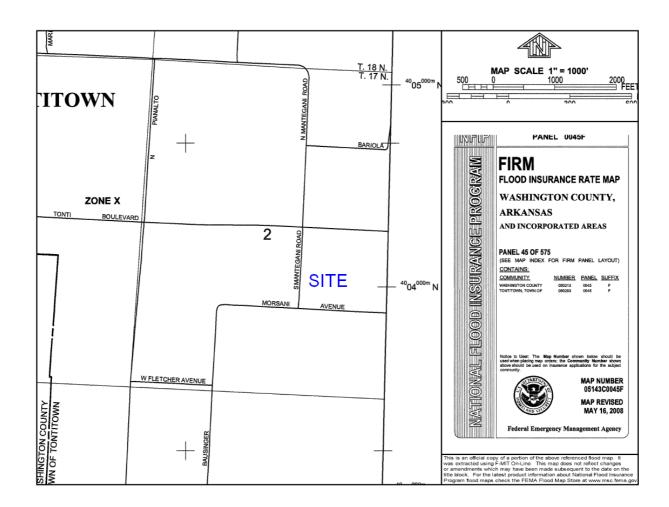
VICINITY MAP



AERIAL PHOTOGRAPH

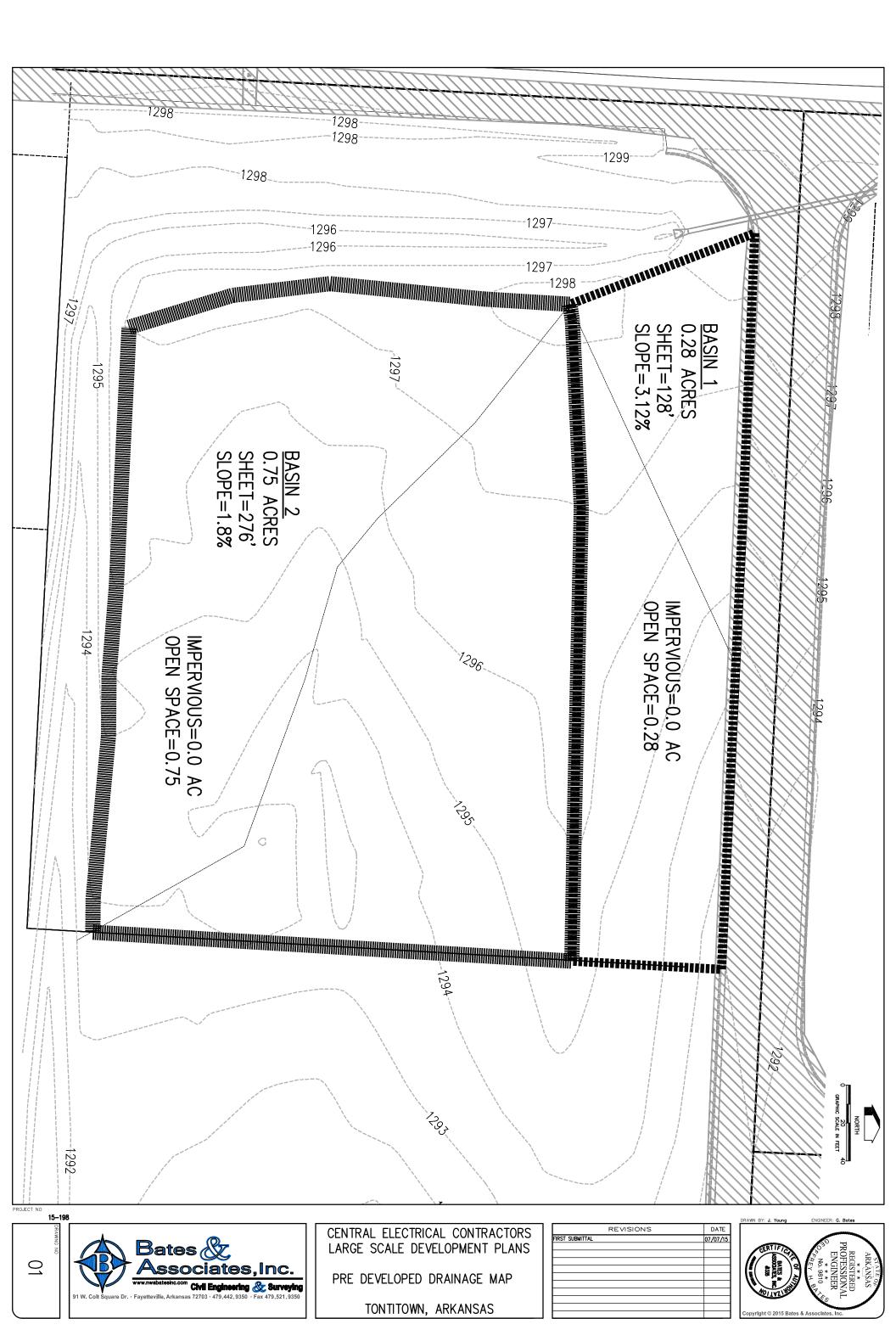


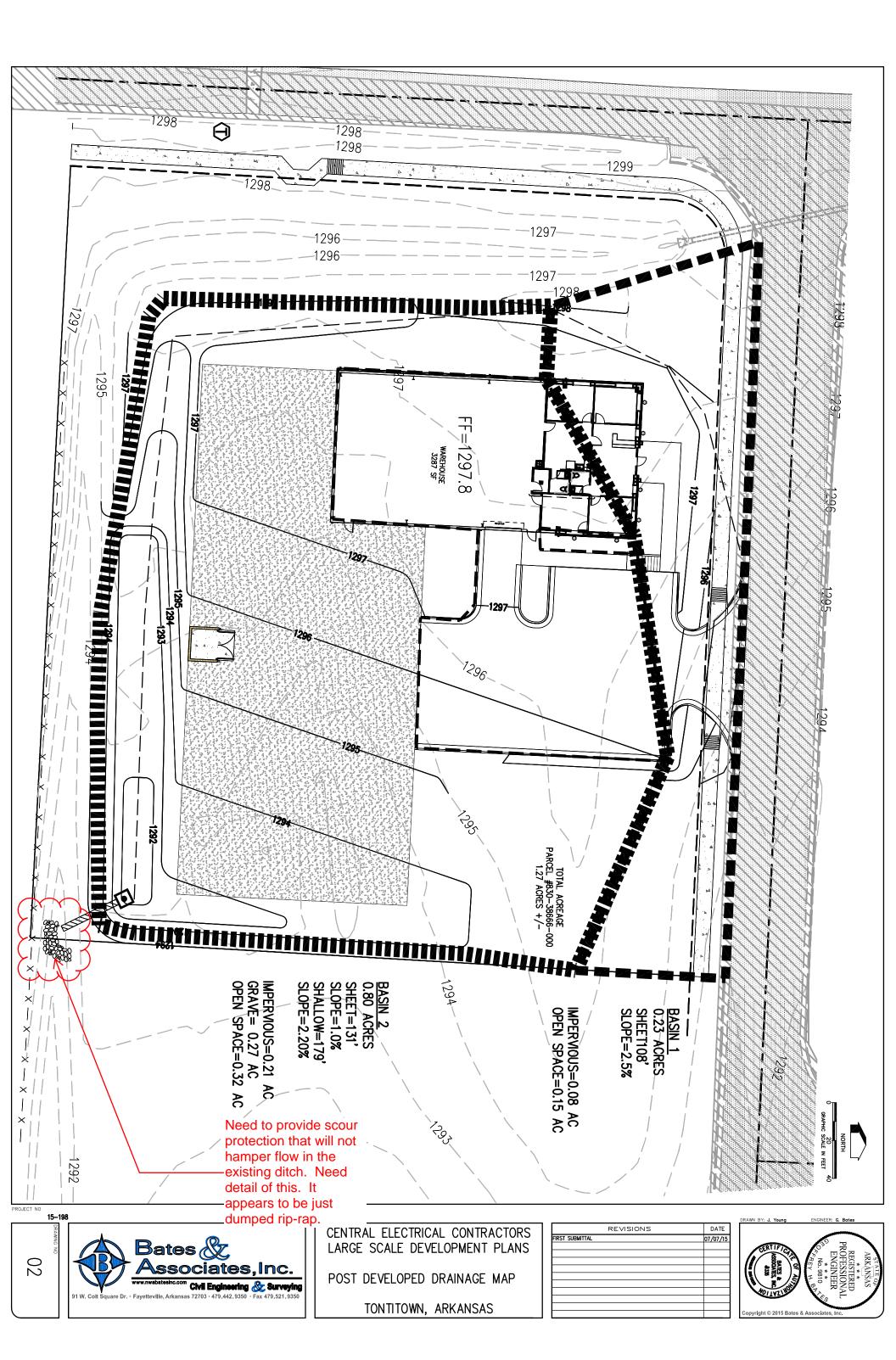
SOILS MAP



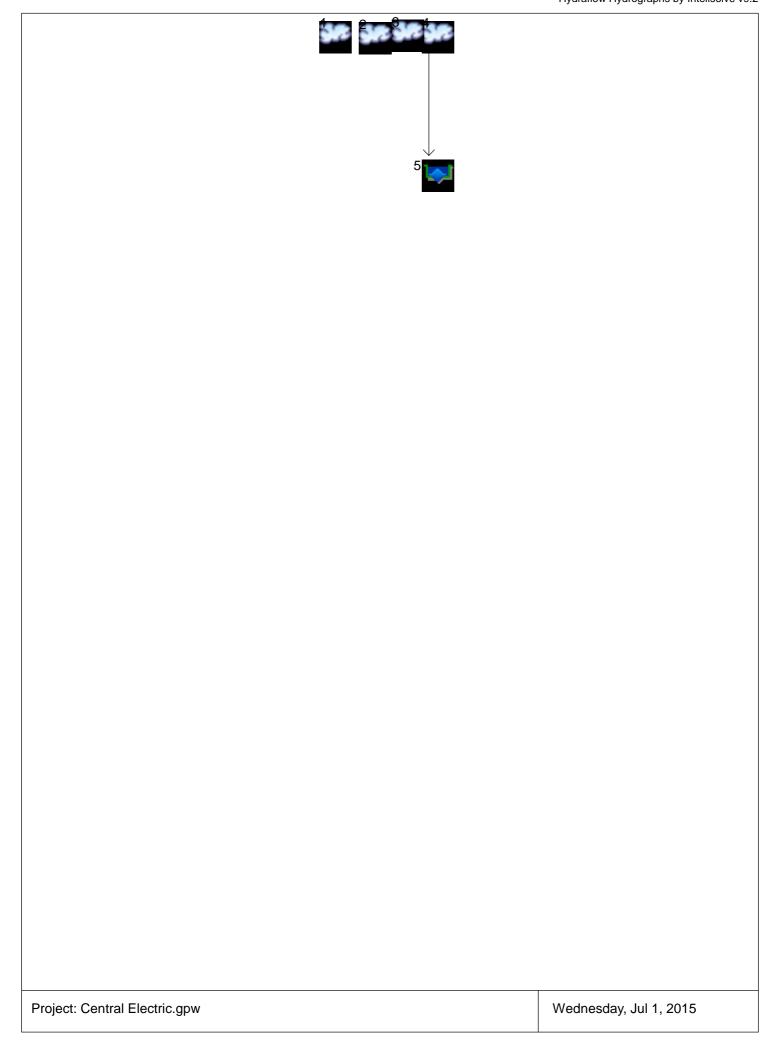
FEMA FIRM PANEL

RUNOFF CALCULATIONS





Watershed Model Schematic



Hydrograph Return Period Recap

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. Hydrograph	Inflow							Hydraflow Hydrographs by Intelisol		Hydrograph
No. type (origin)	Hyd(s)	1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	description
1 SCS Runoff			0.745		0.888	1.228	1.470	1.710	1.890	Pre Developed Basin 1
2 SCS Runoff			1.593		1.878	2.551	3.026	3.499	3.852	Pre Developed Basin 2
3 SCS Runoff			0.685		0.803	1.081	1.276	1.471	1.617	Post Developed Basin 1
4 SCS Runoff			2.026		2.367	3.171	3.739	4.303	4.725	Post Developed Basin 2
5 Reservoir	4		1.569		1.766	2.094	2.359	3.128	3.670	Detention Pond

Proj. file: Central Electric.gpw

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Hydraflow Hydrographs by Intelisolve v9.2

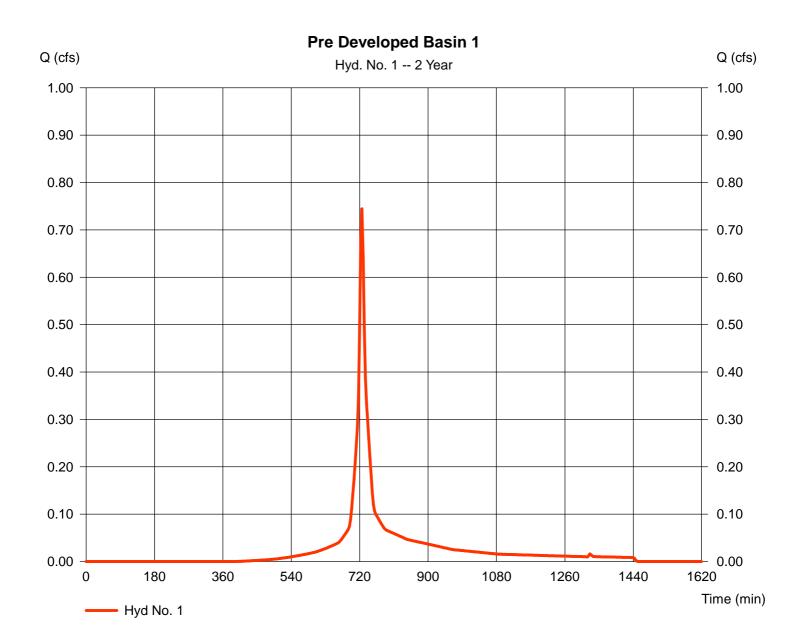
Wednesday, Jul 1, 2015

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 3 minDrainage area = 0.280 acBasin Slope = 0.0 %**TR55** Tc method Total precip. 4.08 in 24 hrs Storm duration

Peak discharge = 0.745 cfsTime to peak = 726 min Hyd. volume = 2.495 cuftCurve number = 86 Hydraulic length = 0 ftTime of conc. (Tc) 8.80 min Distribution <u></u>
→ Type III = 484 Shape factor



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1Pre Developed Basin 1

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 128.0 = 4.08 = 3.12		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.85	+	0.00	+	0.00	=	8.85
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved = 0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs by Intelisolve v9.2

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Hyd. No. 2

Pre Developed Basin 2

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 3 minDrainage area = 0.750 acBasin Slope = 0.0 %Tc method Total precip. 😝 4.08 in = 24 hrs Storm duration

Peak discharge = 1.593 cfs
Time to peak = 735 min
Hyd. volume = 7,693 cuft
Curve number = 89
Hydraulic length = 0 ft

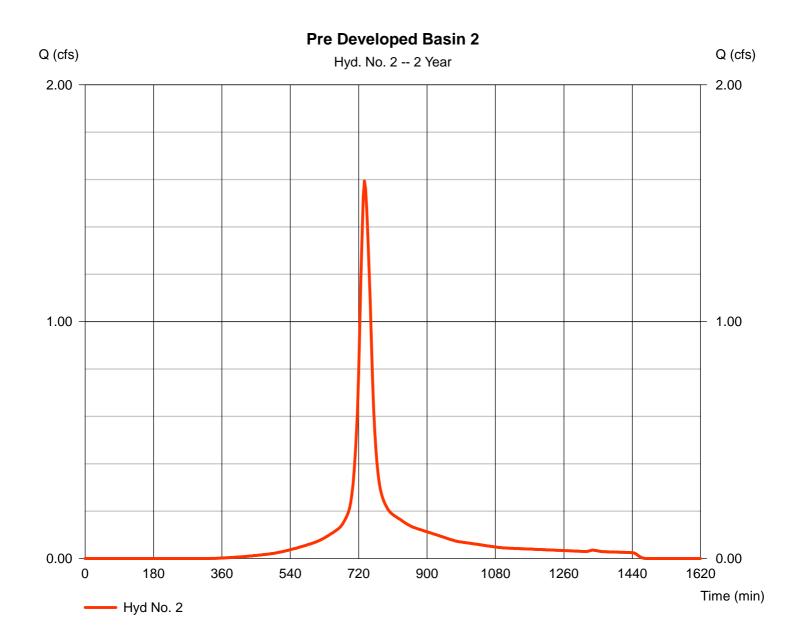
Time of conc. (Tc)

Distribution

Shape factor

20.40 min

Type III



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2Pre Developed Basin 2

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 276.0 = 4.08 = 1.80		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 20.39	+	0.00	+	0.00	=	20.39
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved = 0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							20.40 miı

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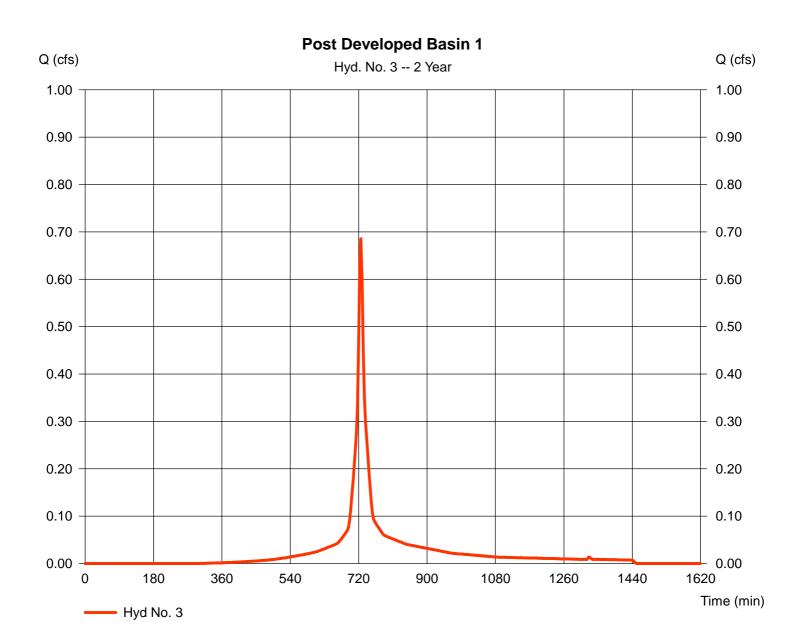
Hyd. No. 3

Post Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 3 min= 0.230 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 4.08 inStorm duration = 24 hrs

Peak discharge = 0.685 cfs
Time to peak = 726 min
Hyd. volume = 2,344 cuft
Curve number = 90*
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.40 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = $[(0.080 \times 98) + (0.150 \times 86)] / 0.230$



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Hyd. No. 3Post Developed Basin 1

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 108.0 = 4.08 = 2.50		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.44	+	0.00	+	0.00	=	8.44
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved = 0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.40 min

Hydraflow Hydrographs by Intelisolve v9.2

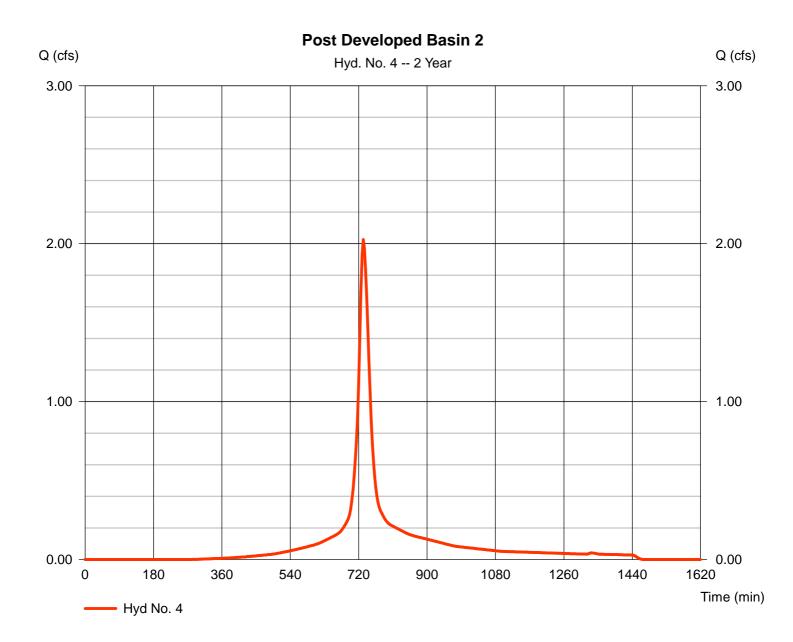
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Hyd. No. 4

Post Developed Basin 2

Hydrograph type = SCS Runoff Peak discharge = 2.026 cfsStorm frequency = 2 yrsTime to peak = 732 min Time interval = 3 minHyd. volume = 9.266 cuft= 91* Drainage area = 0.800 acCurve number Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 minTotal precip. = 4.08 inDistribution = Type III = 484 Storm duration = 24 hrs Shape factor

^{*} Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4Post Developed Basin 2

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 131.0 = 4.08 = 1.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 14.21	+	0.00	+	0.00	=	14.21
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 179.00 = 2.20 = Unpave = 2.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.25	+	0.00	+	0.00	=	1.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.50 mir

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= 1.569 cfs

= 744 min

Hyd. No. 5

Detention Pond

Hydrograph type = Reservoir Storm frequency = 2 yrs Time interval = 3 min

Inflow hyd. No. = 4 - Post Developed Basin 2

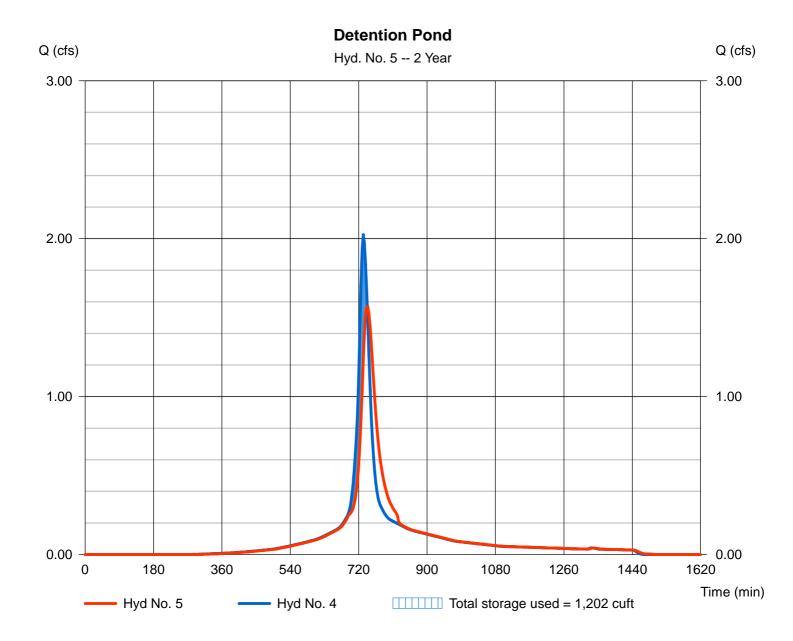
Reservoir name = <New Pond>

Hyd. volume = 9,264 cuft
Max. Elevation = 1292.93 ft
Max. Storage = 1,202 cuft

Peak discharge

Time to peak

Storage Indication method used.



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Pond No. 1 - <New Pond>

Pond Data

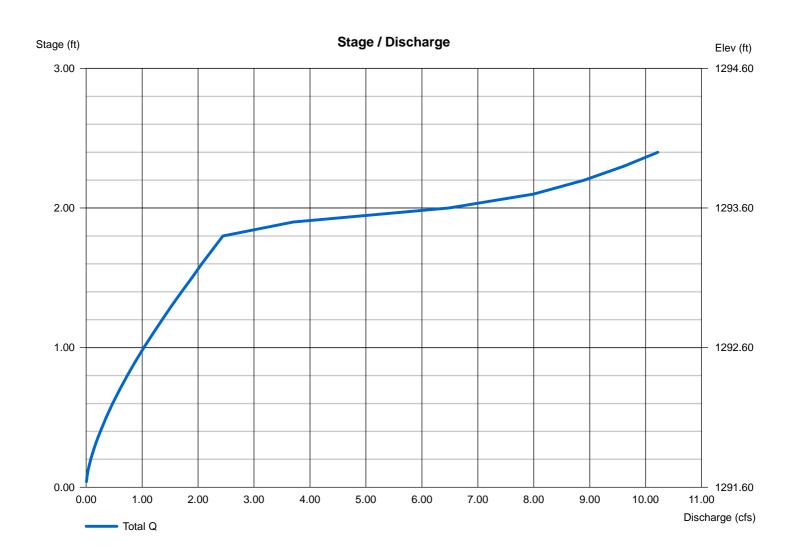
Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 1291.60 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	1291.60	10	0	0
0.40	1292.00	412	65	65
1.40	1293.00	2,308	1,232	1,296
2.40	1294.00	6,245	4,116	5,412

Culvert / Orifice Structures Weir Structures [B] [C] [PrfRsr] [A] [C] [D] [A] [B] Rise (in) = 18.00 0.00 0.00 0.00 = 16.00 0.38 0.00 0.00 Crest Len (ft) Span (in) = 18.00 0.00 Crest El. (ft) 0.00 0.00 = 1293.401291.61 0.00 0.00 No. Barrels = 1 0 0 Weir Coeff. = 3.333.33 3.33 3.33 0 Invert El. (ft) = 1291.60 0.00 0.00 0.00 Weir Type = Riser Rect = 20.000.00 0.00 0.00 Multi-Stage Length (ft) = Yes Yes No No = 0.500.00 0.00 n/a Slope (%) N-Value = .013 .013 .013 n/a = 0.000 (by Wet area) Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) Multi-Stage = n/aNo No No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



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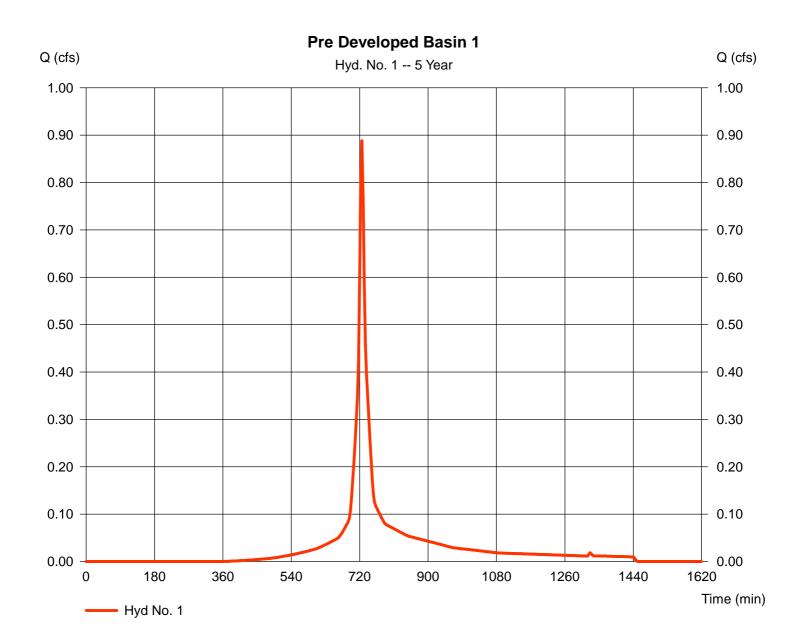
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Hyd. No. 1

Pre Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 3 minDrainage area = 0.280 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 4.65 inStorm duration = 24 hrs

Peak discharge = 0.888 cfs
Time to peak = 726 min
Hyd. volume = 2,994 cuft
Curve number = 86
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type III
Shape factor = 484



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= 1.878 cfs

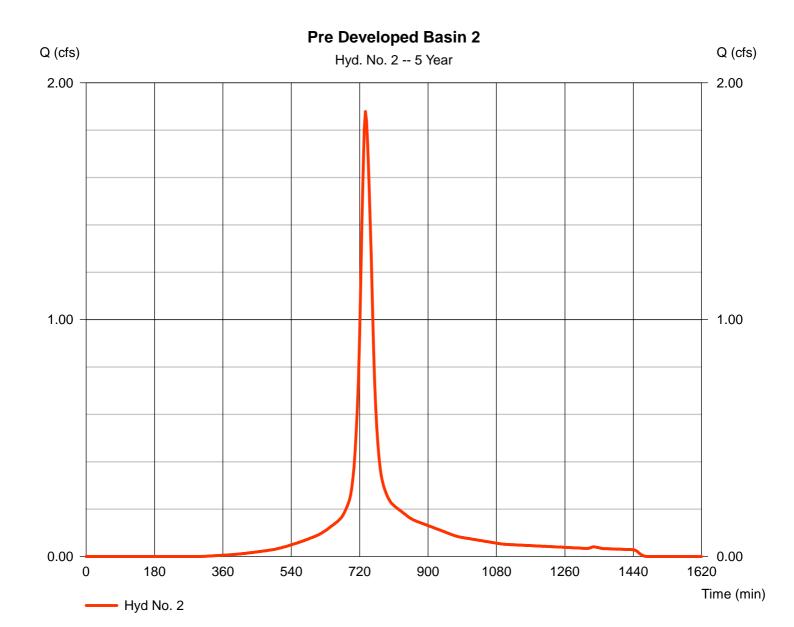
Hyd. No. 2

Pre Developed Basin 2

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 3 min= 0.750 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 4.65 inStorm duration = 24 hrs

Time to peak = 735 min
Hyd. volume = 9,125 cuft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 20.40 min
Distribution = Type III
Shape factor = 484

Peak discharge



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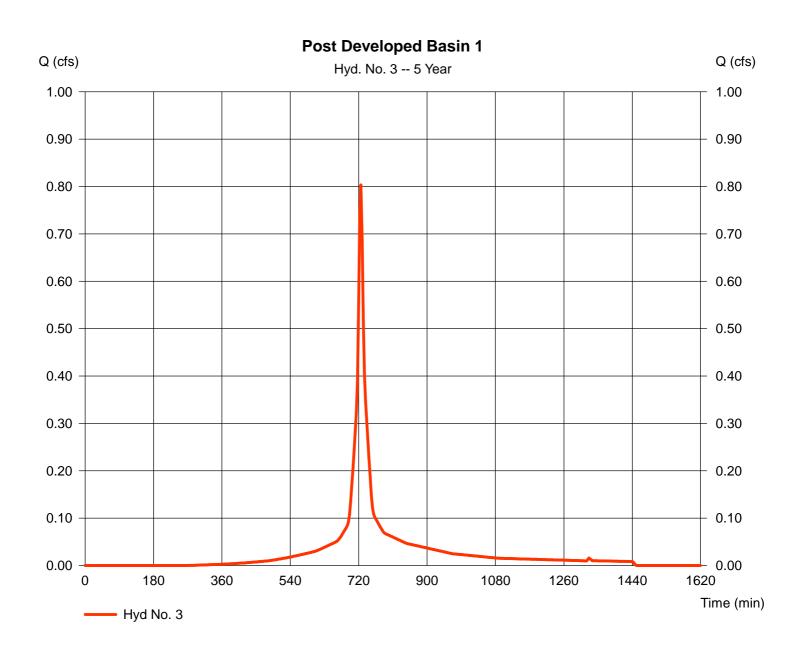
Wednesday, Jul 1, 2015

Hyd. No. 3

Post Developed Basin 1

Hydrograph type = SCS Runoff Peak discharge = 0.803 cfsStorm frequency = 5 yrsTime to peak = 726 min Time interval = 3 minHyd. volume = 2.770 cuft= 0.230 acCurve number = 90*Drainage area Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 8.40 minDistribution Total precip. = 4.65 in= Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.080 \times 98) + (0.150 \times 86)] / 0.230$



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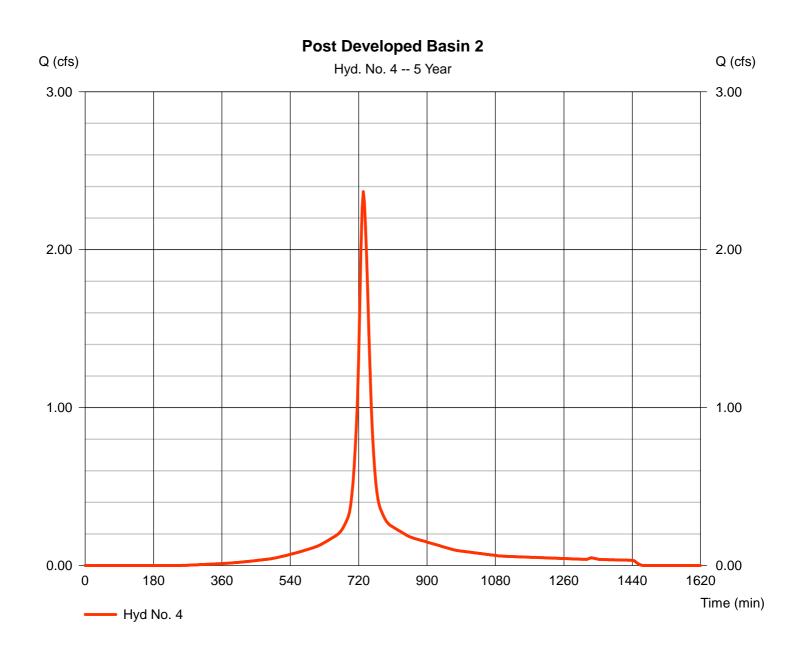
Wednesday, Jul 1, 2015

Hyd. No. 4

Post Developed Basin 2

Hydrograph type = SCS Runoff Peak discharge = 2.367 cfsStorm frequency = 5 yrsTime to peak = 732 min Time interval = 3 minHyd. volume = 10.910 cuft= 91* Drainage area = 0.800 acCurve number Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 minTotal precip. = 4.65 inDistribution = Type III = 484 Storm duration = 24 hrs Shape factor

^{*} Composite (Area/CN) = $[(0.320 \times 89) + (0.270 \times 89) + (0.210 \times 98)] / 0.800$



Hydraflow Hydrographs by Intelisolve v9.2

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= 1.766 cfs

= 10,908 cuft

= 1293.04 ft

= 1,434 cuft

= 744 min

Peak discharge

Time to peak

Hyd. volume

Max. Elevation

Max. Storage

Hyd. No. 5

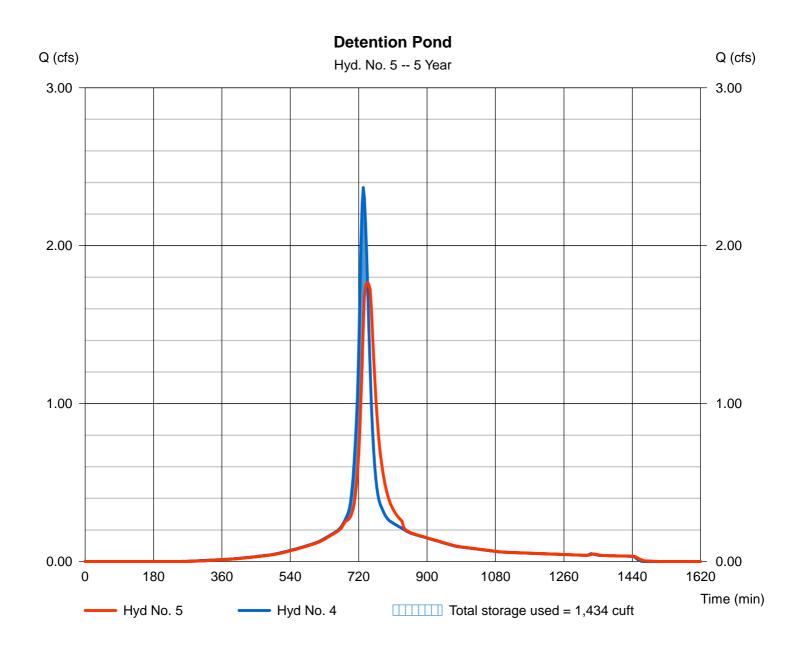
Detention Pond

Hydrograph type = Reservoir Storm frequency = 5 yrs Time interval = 3 min

Inflow hyd. No. = 4 - Post Developed Basin 2

Reservoir name = <New Pond>

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.2

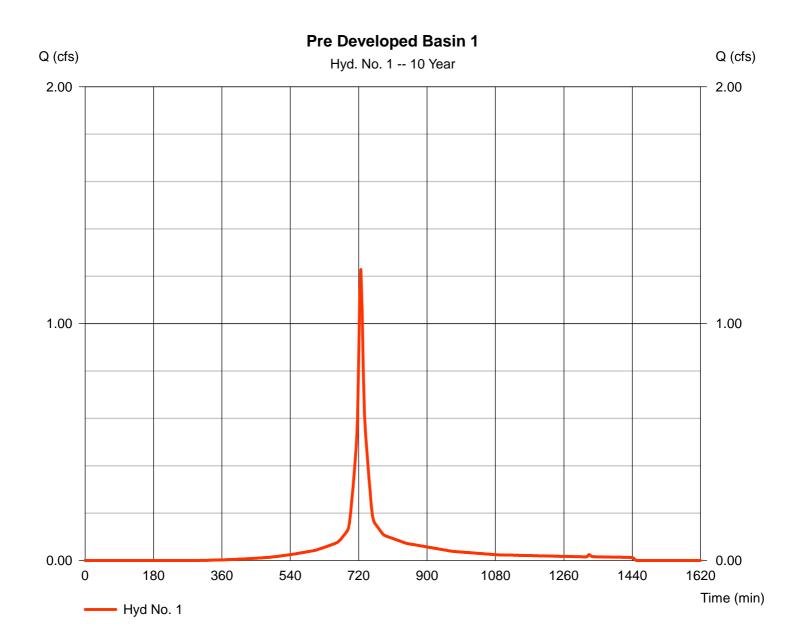
Wednesday, Jul 1, 2015

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 3 min= 0.280 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 6.00 inStorm duration = 24 hrs

Peak discharge = 1.228 cfs
Time to peak = 726 min
Hyd. volume = 4,202 cuft
Curve number = 86
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.2

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Hyd. No. 2

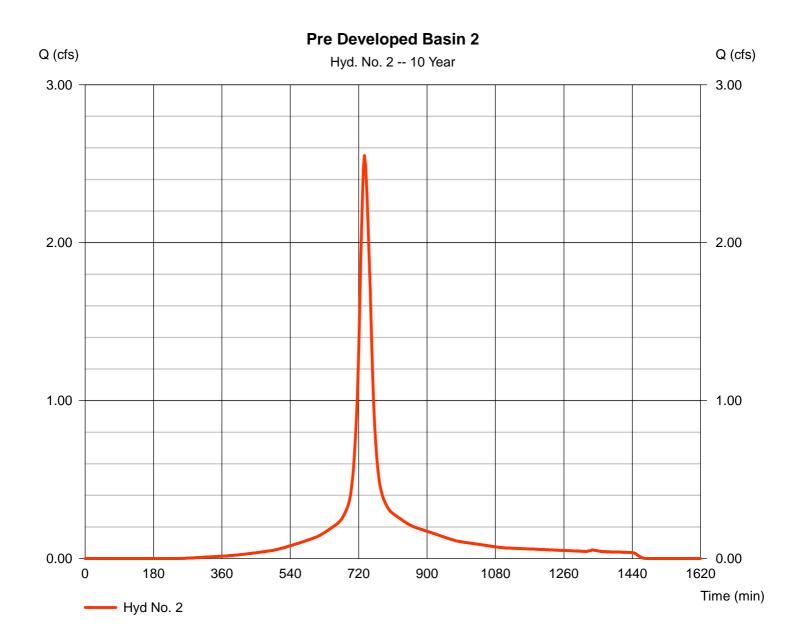
Pre Developed Basin 2

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 3 min= 0.750 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 6.00 in= 24 hrs Storm duration

Peak discharge = 2.551 cfs
Time to peak = 735 min
Hyd. volume = 12,570 cuft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 20.40 min
Distribution = Type III

Shape factor

= 484



Hydraflow Hydrographs by Intelisolve v9.2

Wednesday, Jul 1, 2015

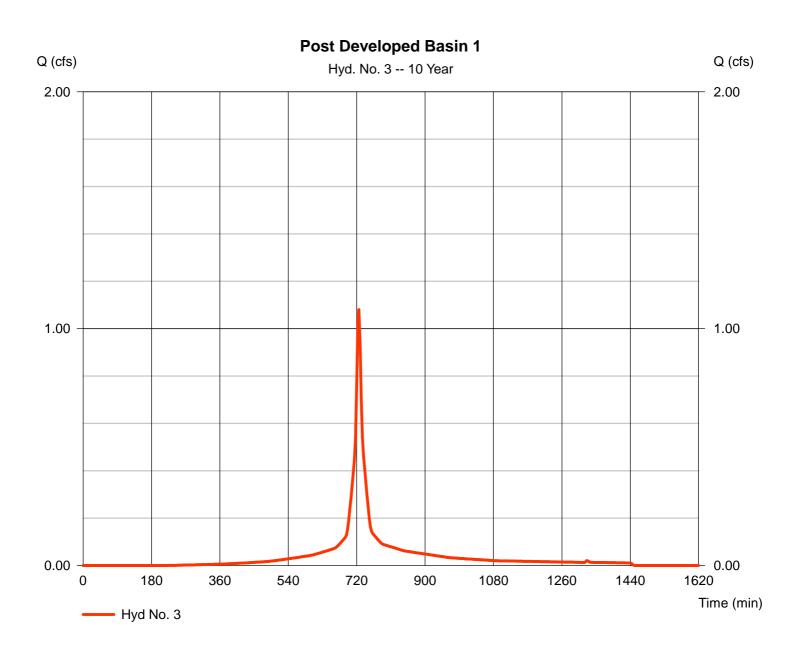
Hyd. No. 3

Post Developed Basin 1

Hydrograph type = SCS Runoff Peak discharge Storm frequency = 10 yrsTime interval = 3 min= 0.230 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 6.00 inStorm duration = 24 hrs

= 1.081 cfsTime to peak = 726 min Hyd. volume = 3.793 cuft= 90*Curve number Hydraulic length = 0 ftTime of conc. (Tc) = 8.40 minDistribution = Type III Shape factor = 484

^{*} Composite (Area/CN) = $[(0.080 \times 98) + (0.150 \times 86)] / 0.230$



Hydraflow Hydrographs by Intelisolve v9.2

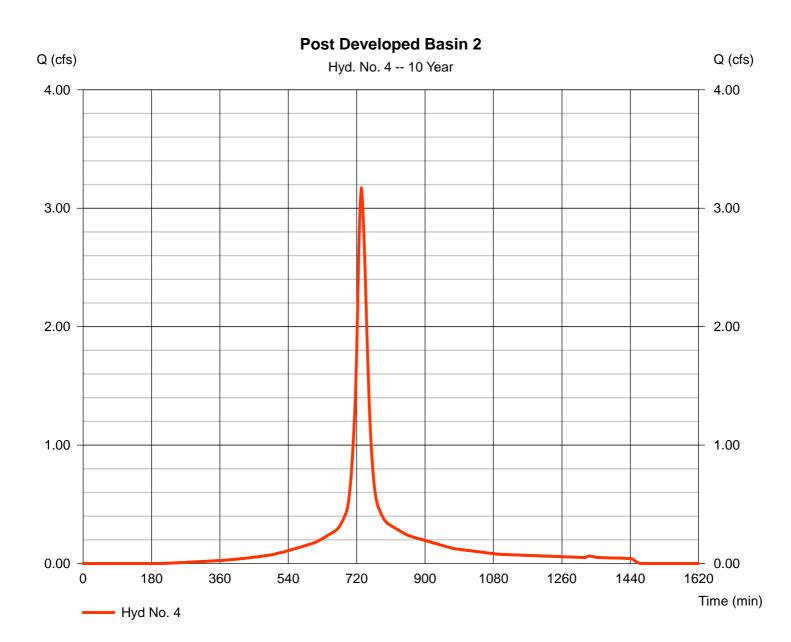
Wednesday, Jul 1, 2015

Hyd. No. 4

Post Developed Basin 2

Hydrograph type = SCS Runoff Peak discharge = 3.171 cfsStorm frequency = 10 yrsTime to peak = 732 min Time interval = 3 minHyd. volume = 14,846 cuft = 0.800 ac= 91* Drainage area Curve number Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 minDistribution Total precip. = 6.00 in= Type III = 484 Storm duration = 24 hrs Shape factor

^{*} Composite (Area/CN) = $[(0.320 \times 89) + (0.270 \times 89) + (0.210 \times 98)] / 0.800$



Hydraflow Hydrographs by Intelisolve v9.2

Wednesday, Jul 1, 2015

= 2.094 cfs

= 14,844 cuft

= 747 min

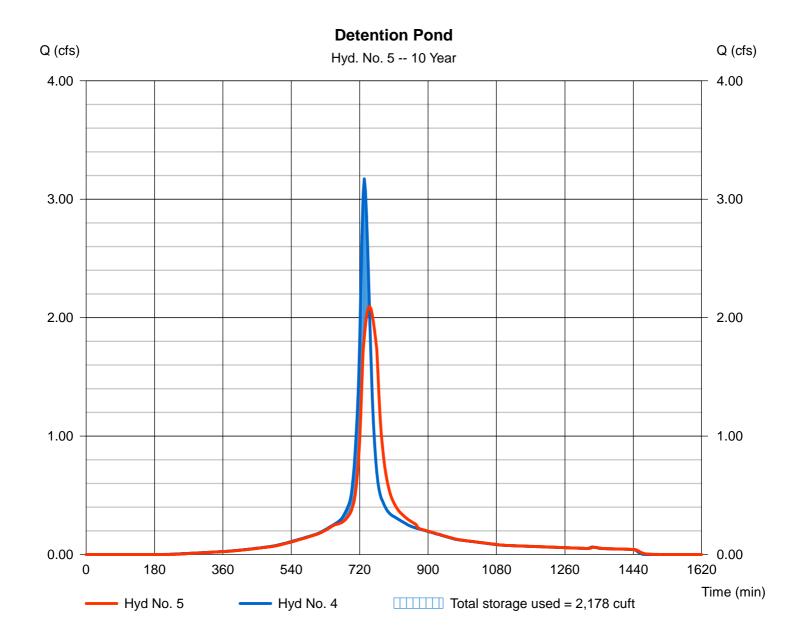
Hyd. No. 5

Detention Pond

Hydrograph type = Reservoir Peak discharge
Storm frequency = 10 yrs Time to peak
Time interval = 3 min Hyd. volume

Inflow hyd. No. = 4 - Post Developed Basin 2 Max. Elevation = 1293.22 ft Reservoir name = <New Pond> Max. Storage = 2,178 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.2

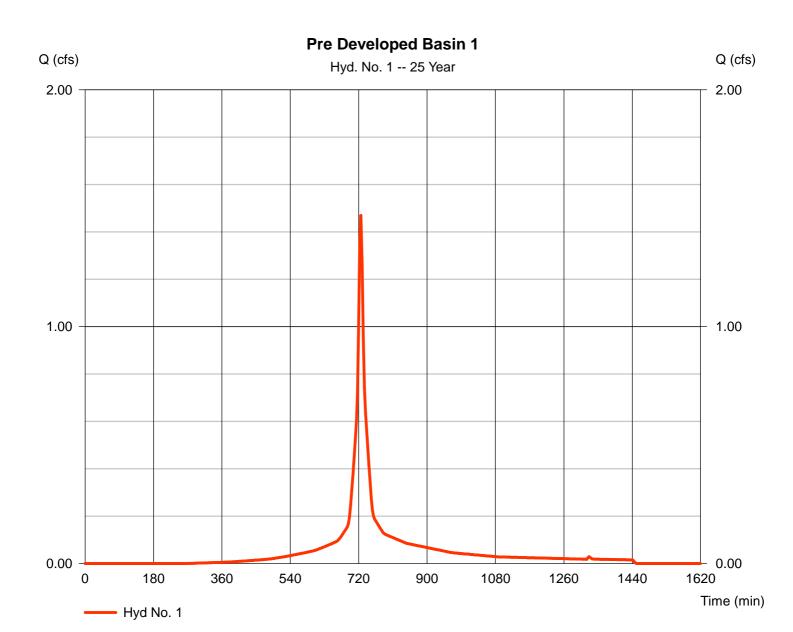
Wednesday, Jul 1, 2015

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 25 yrsTime interval = 3 min= 0.280 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 6.96 inStorm duration = 24 hrs

Peak discharge = 1.470 cfs
Time to peak = 726 min
Hyd. volume = 5,076 cuft
Curve number = 86
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.2

Wednesday, Jul 1, 2015

Hyd. No. 2

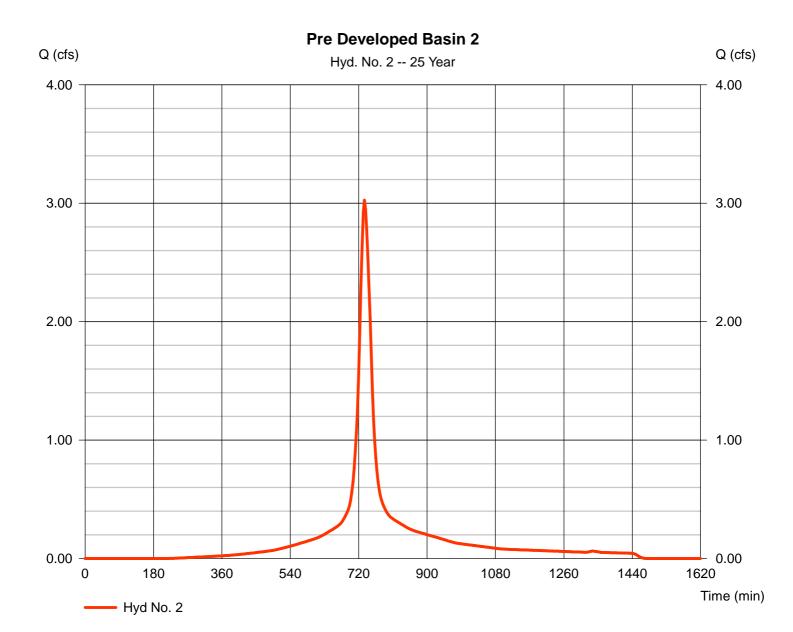
Pre Developed Basin 2

Hydrograph type = SCS Runoff Storm frequency = 25 yrsTime interval = 3 min= 0.750 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 6.96 inStorm duration = 24 hrs

Peak discharge = 3.026 cfs
Time to peak = 735 min
Hyd. volume = 15,048 cuft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 20.40 min
Distribution = Type III

Shape factor

= 484



Hydraflow Hydrographs by Intelisolve v9.2

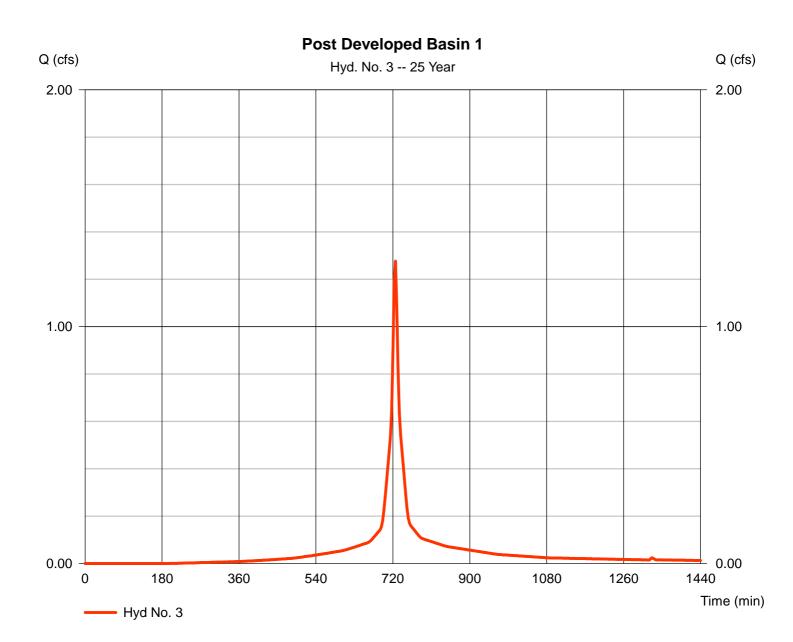
Wednesday, Jul 1, 2015

Hyd. No. 3

Post Developed Basin 1

Hydrograph type = SCS Runoff Peak discharge = 1.276 cfsStorm frequency = 25 yrsTime to peak = 726 min Time interval = 3 minHyd. volume = 4.527 cuft= 0.230 ac= 90*Drainage area Curve number Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 8.40 minTotal precip. = 6.96 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.080 \times 98) + (0.150 \times 86)] / 0.230$



Hydraflow Hydrographs by Intelisolve v9.2

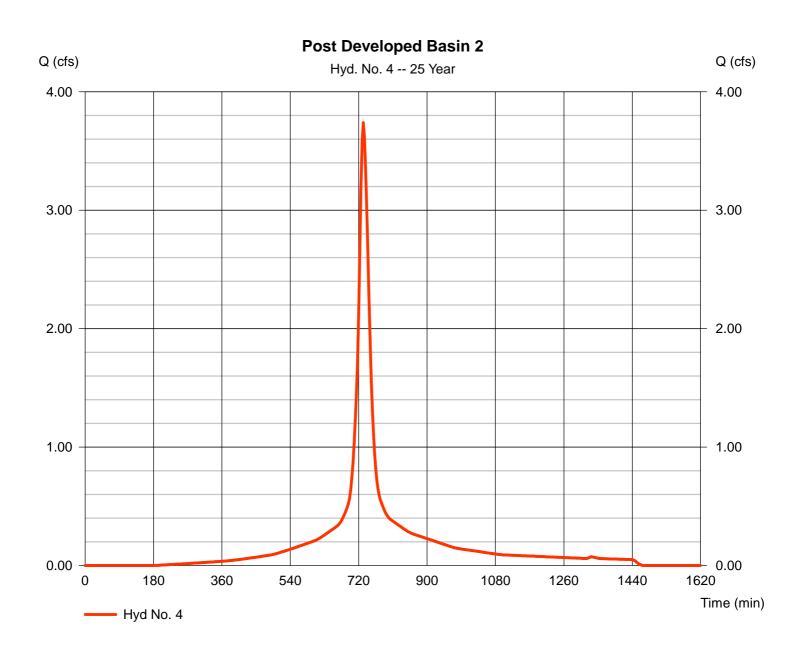
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Hyd. No. 4

Post Developed Basin 2

Hydrograph type = SCS Runoff Peak discharge = 3.739 cfsStorm frequency = 25 yrsTime to peak = 732 min Time interval = 3 minHyd. volume = 17.667 cuft = 0.800 ac= 91* Drainage area Curve number Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 minTotal precip. = 6.96 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.320 \times 89) + (0.270 \times 89) + (0.210 \times 98)] / 0.800$



Hydraflow Hydrographs by Intelisolve v9.2

Wednesday, Jul 1, 2015

= 2.359 cfs

Hyd. No. 5

Detention Pond

Hydrograph type = Reservoir Storm frequency = 25 yrs Time interval = 3 min

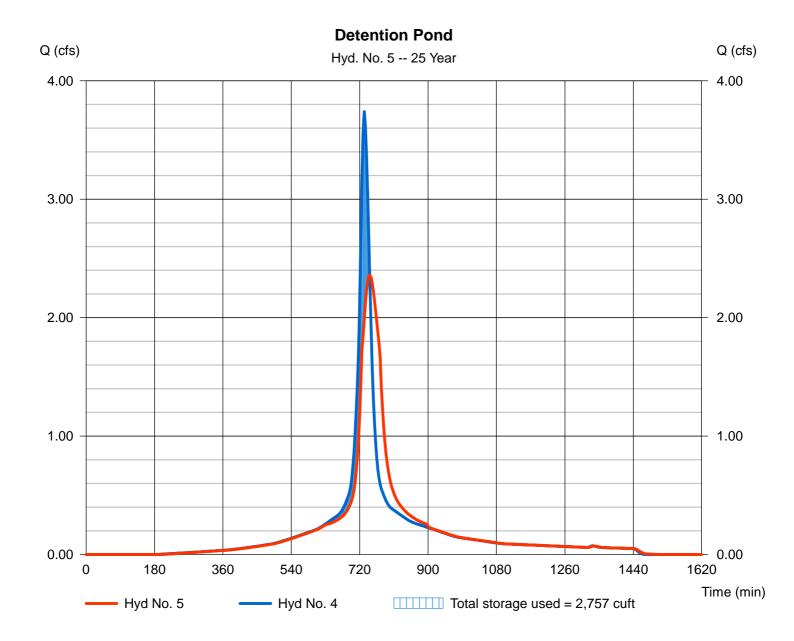
Inflow hyd. No. = 4 - Post Developed Basin 2

Reservoir name = <New Pond>

Time to peak = 747 min
Hyd. volume = 17,666 cuft
Max. Elevation = 1293.36 ft
Max. Storage = 2,757 cuft

Peak discharge

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.2

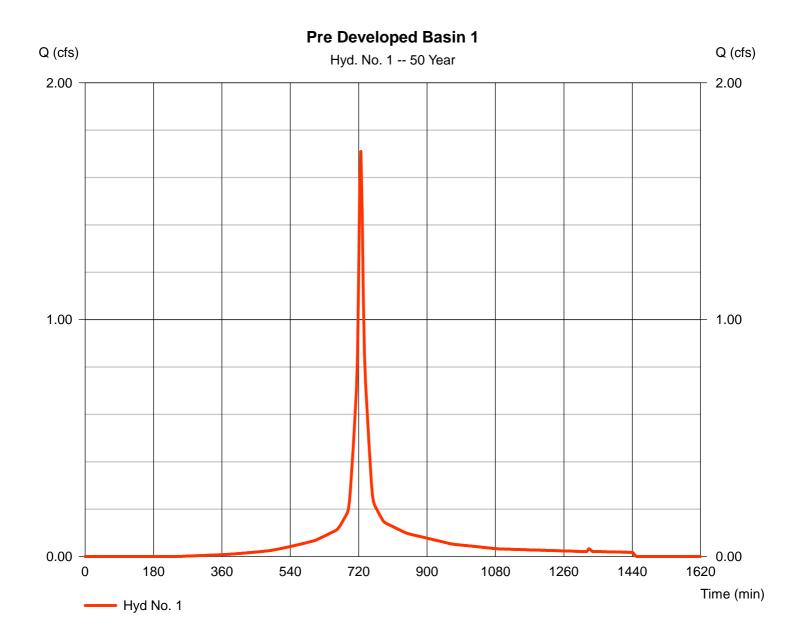
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Hyd. No. 1

Pre Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 3 min= 0.280 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 7.92 inStorm duration = 24 hrs

Peak discharge = 1.710 cfs
Time to peak = 726 min
Hyd. volume = 5,959 cuft
Curve number = 86
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.2

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Hyd. No. 2

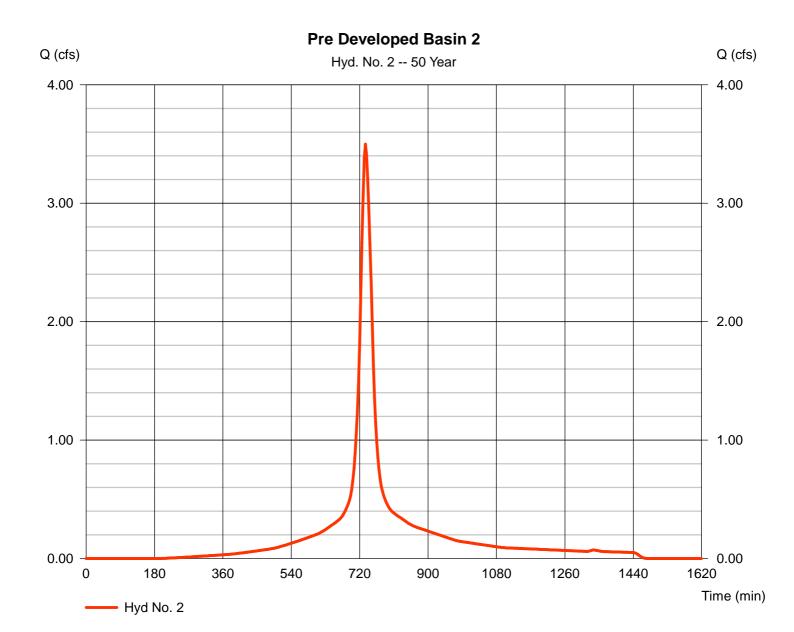
Pre Developed Basin 2

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 3 min= 0.750 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 7.92 inStorm duration = 24 hrs

Peak discharge = 3.499 cfs
Time to peak = 735 min
Hyd. volume = 17,541 cuft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 20.40 min
Distribution = Type III

Shape factor

= 484



Hydraflow Hydrographs by Intelisolve v9.2

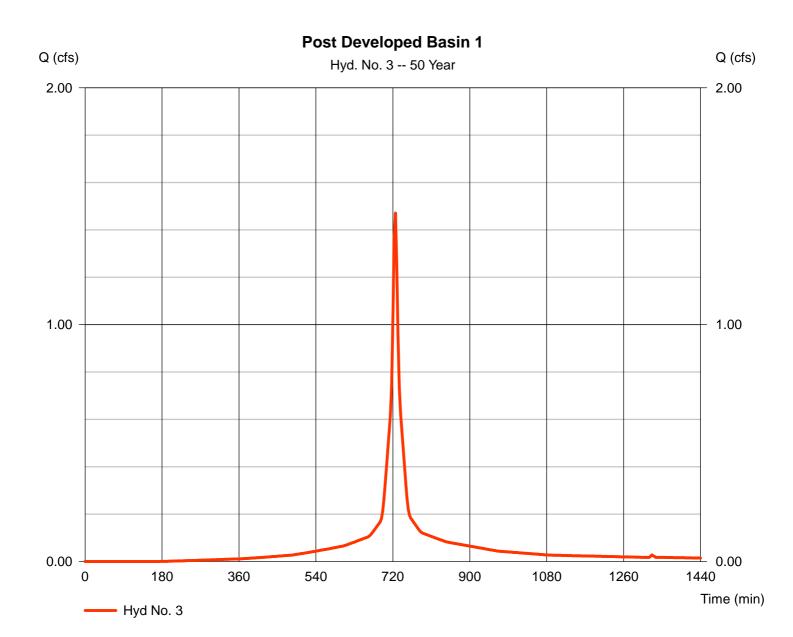
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Hyd. No. 3

Post Developed Basin 1

Hydrograph type = SCS Runoff Peak discharge = 1.471 cfsStorm frequency = 50 yrsTime to peak = 726 min Time interval = 3 minHyd. volume = 5.265 cuft= 0.230 ac= 90*Drainage area Curve number Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 8.40 minTotal precip. = 7.92 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.080 \times 98) + (0.150 \times 86)] / 0.230$



Hydraflow Hydrographs by Intelisolve v9.2

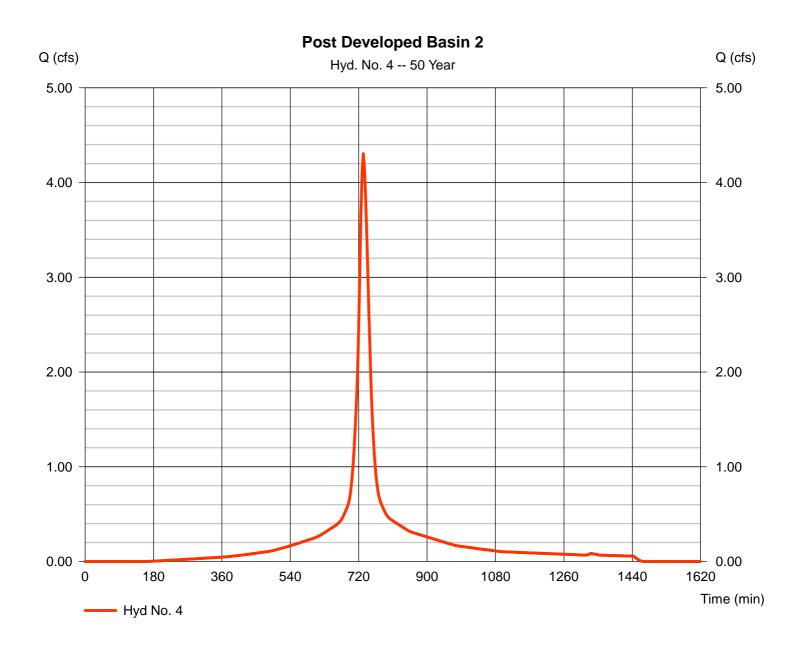
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Hyd. No. 4

Post Developed Basin 2

Hydrograph type = SCS Runoff Peak discharge = 4.303 cfsStorm frequency = 50 yrsTime to peak = 732 min Time interval = 3 minHyd. volume = 20,500 cuftDrainage area = 0.800 acCurve number = 91*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 minTotal precip. = 7.92 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.320 \times 89) + (0.270 \times 89) + (0.210 \times 98)] / 0.800$



Hydraflow Hydrographs by Intelisolve v9.2

Wednesday, Jul 1, 2015

= 3.128 cfs

= 20.499 cuft

= 744 min

Peak discharge

Time to peak

Hyd. volume

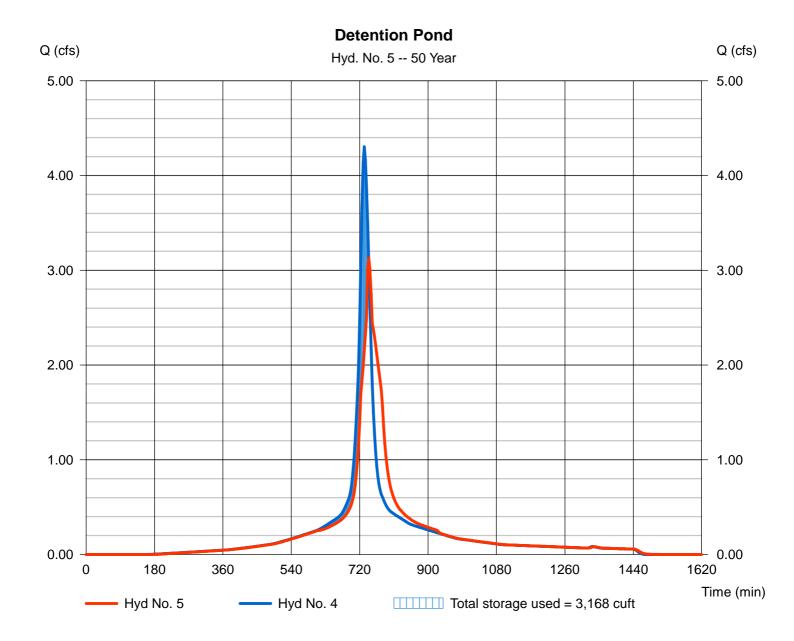
Hyd. No. 5

Detention Pond

Hydrograph type = Reservoir
Storm frequency = 50 yrs
Time interval = 3 min

Inflow hyd. No. = 4 - Post Developed Basin 2 Max. Elevation = 1293.46 ft Reservoir name = <New Pond> Max. Storage = 3,168 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.2

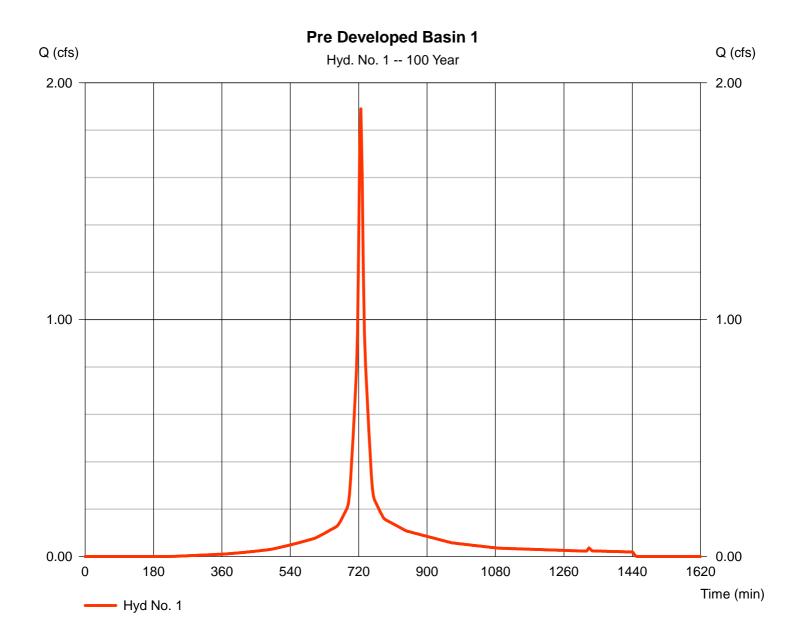
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Hyd. No. 1

Pre Developed Basin 1

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval $= 3 \min$ = 0.280 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 8.64 inStorm duration = 24 hrs

Peak discharge = 1.890 cfs
Time to peak = 726 min
Hyd. volume = 6,625 cuft
Curve number = 86
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.2

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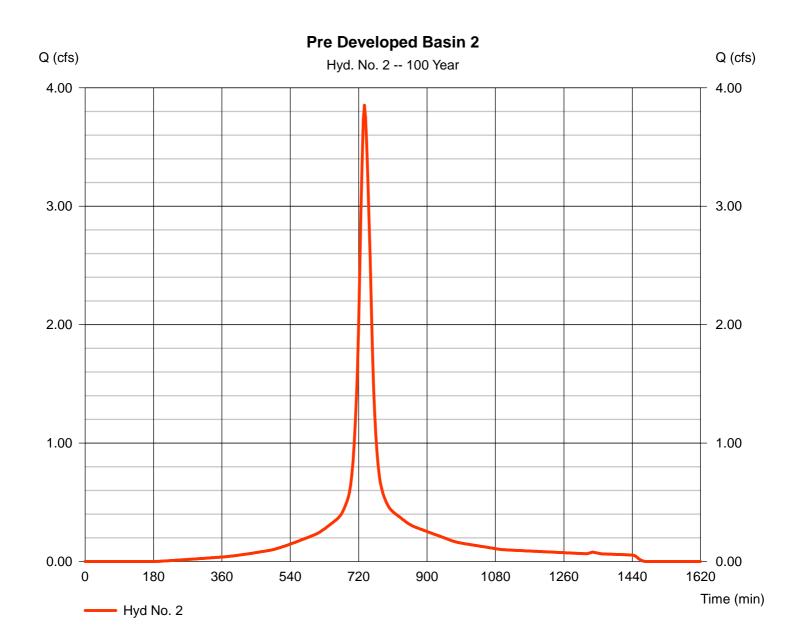
Hyd. No. 2

Pre Developed Basin 2

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval $= 3 \min$ = 0.750 acDrainage area Basin Slope = 0.0 %Tc method = TR55 Total precip. = 8.64 inStorm duration = 24 hrs

Peak discharge = 3.852 cfs
Time to peak = 735 min
Hyd. volume = 19,419 cuft
Curve number = 89
Hydraulic length = 0 ft

Time of conc. (Tc) = 20.40 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.2

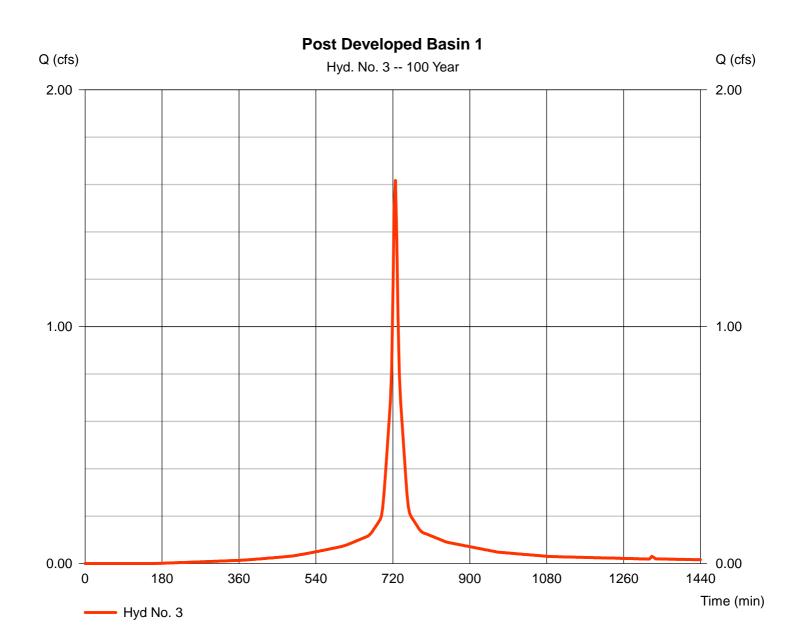
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Hyd. No. 3

Post Developed Basin 1

= 1.617 cfsHydrograph type = SCS Runoff Peak discharge Storm frequency = 100 yrsTime to peak = 726 min Time interval $= 3 \min$ Hyd. volume = 5.820 cuft= 0.230 ac= 90*Drainage area Curve number Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 8.40 minTc method = TR55 Total precip. = 8.64 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.080 \times 98) + (0.150 \times 86)] / 0.230$



Hydraflow Hydrographs by Intelisolve v9.2

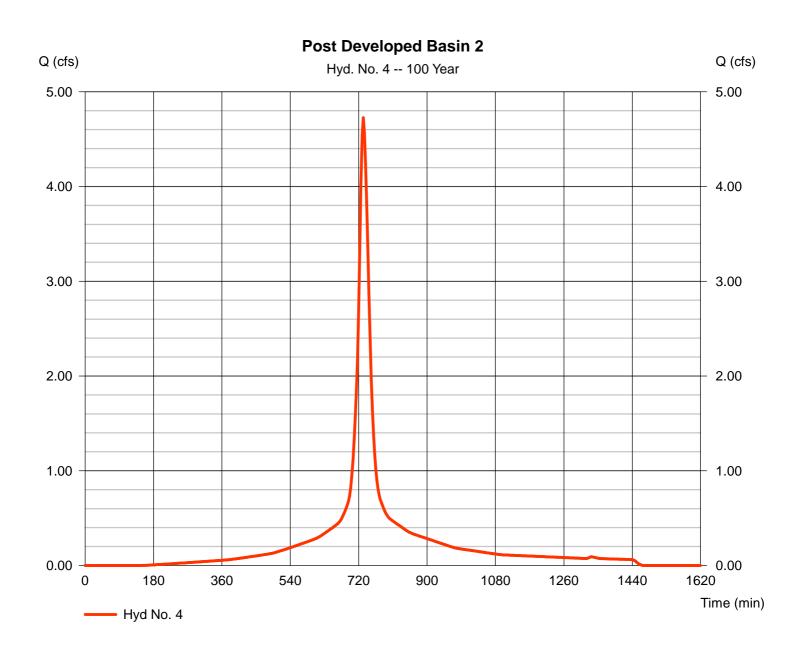
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Hyd. No. 4

Post Developed Basin 2

Hydrograph type = SCS Runoff Peak discharge = 4.725 cfsStorm frequency = 100 yrsTime to peak = 732 min Time interval = 3 minHyd. volume = 22,631 cuft Drainage area = 0.800 acCurve number = 91*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 minTotal precip. = 8.64 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.320 \times 89) + (0.270 \times 89) + (0.210 \times 98)] / 0.800$



Hydraflow Hydrographs by Intelisolve v9.2

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= 3.670 cfs

= 22,630 cuft

= 1293.50 ft

= 741 min

Peak discharge

Myd volume Y

Max. Elevation

Time to peak

Hyd. No. 5

Detention Pond

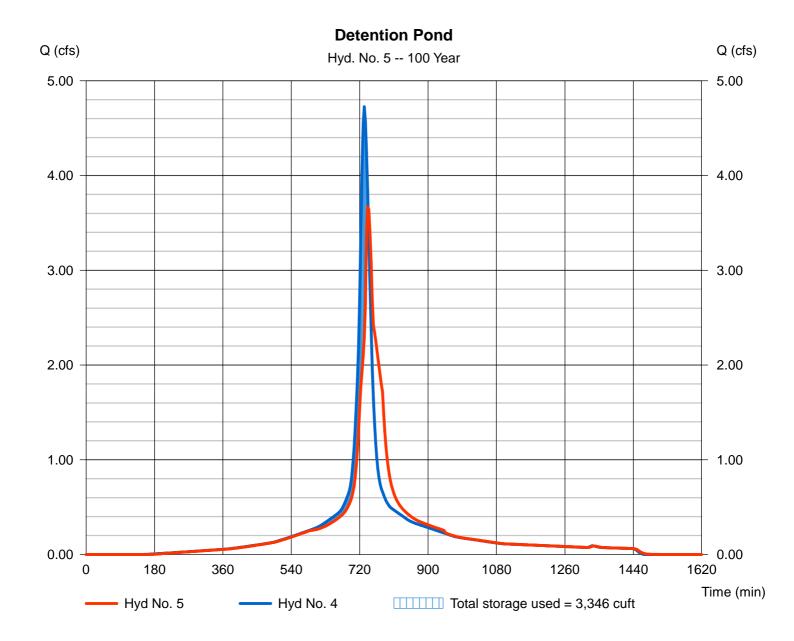
Hydrograph type = Reservoir Storm frequency = 100 yrsTime interval = 3 min

= 4 - Post Developed Basin 2 Inflow hyd. No.

= <New Pond> Reservoir name

Max. Storage = 3,346 cuft

Storage Indication method used.



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