

91 W. Colt Square Dr. Suite 3 / Fayetteville, AR /2/0 PH: 479-442-9350 * FAX: 479-521-9350

DRAINAGE DESIGN COMPUTATIONS

For

CENTRAL ELECTRIC BA No. 15-198

FLORENCE AVENUE TONTITOWN, ARKANSAS

JULY 7, 2015

SUBMITTED TO: CITY OF TONTITOWN

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DRAINAGE LETTER

VICINITY MAP

AERIAL PHOTOGRAPH

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PRE AND POST DRAINAGE AREA MAP

RUNOFF CALCULATIONS

REFERENCES

PROJECT OWNER AND DEVELOPER:

Central Electric Contractors 228 S. 40th Street A Springdale, AR 72756

PROJECT TITLE:

The following information is the drainage report for Central Electric.

PROJECT LOCATION:

This project is located at 1167 Florence Ave, in Tontitown, Arkansas in Washington County. See the attached vicinity map for the exact location.

PROJECT DESCRIPTION:

This project site is a 1.27 acre tract in the existing Tontitown Plaza subdivision. Improvements to the site will consist of paving parking lot, drives and adding a 5,000 s.f. building. See the large scale development plan for details.

SITE DRAINAGE:

This project is a small part of a large drainage basin that flows into Brush Creek and eventually discharging into the Illinois River. The soil type for the drainage basin found in the Natural Resources Conservation Service Soil Survey is Captina silt loam which belongs to Hydrologic Soil Group C and Johnsburg which belongs to Hydrologic Soil Group D. The majority of the soil types in the basin are in Soil Group C.

Soil group C soils have low infiltration rates when thoroughly wetted and consist chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine to fine texture. These soils have a low rate of water transmission.

Group D soils have very low infiltration rates when thoroughly wetted and consist chiefly of clay soils with a high swelling potential, soils with a permanent high water table, soils with a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. These soils have a very low rate of water transmission (0-0.05 in/hr).

No portion of this property is located within Flood Zone "AE" and the "Floodway" as determined by the National Flood Insurance Program's Flood Insurance Rate Map for Washington County, Arkansas (Map No. 05143C0045 F, May 16, 2008).

Currently, the runoff from this site sheet flows from the northwest to southeast. Once developed, the runoff from the site will continue in the same general pattern.

AREA DRAIANGE PROBLEMS:

To our knowledge, there are no known drainage problems in this area at this time.

DRAINAGE DESIGN:

Runoff Coefficients for each drainage basin was selected from the Tontitown Drainage Manual. The coefficient was selected based on the amount of pervious and impervious area in the basin. A composite runoff curve number was then calculated for the basin. The composite runoff coefficients are as follows:

	(50%)	(50%)
Cover	Type C	Type D
Open Space (good cove	r) 86	89
Gravel	89	
Impervious	98	98
Pre Developed Basin	1	(0.28 ac)
86 Open space	e	0.28 ac
Pre Developed Basin	2	(0.75 ac)
89 Open space	e	0.75 ac
Post Developed Basin	n 1	(0.23 ac)
86 Open space		0.08 ac
98 Imperviou	S	0.15 ac
I		

90 Post developed composite curve number

Post Dev	veloped Basin 2	(0.80 ac)
89	Open space	0.32 ac
89	Gravel	0.27 ac
98	Impervious	0.21 ac

91 Post developed composite curve number

Basin 1 Runoff Calculations

The 2-year through 100-year frequency storm events for pre- and post-development flows were calculated using the drainage program Hydraflow Hydrographs (SCS Method). The post developed peak flows decreased slightly due to decreasing the basin size.

Basin 1:

The peak runoff will slightly increase in basin 1 due to the increase in impervious area.

	(Peak flow in cfs)									
Design										
Storm	Pre-Dev.	Post-Dev	Difference							
2-yr	0.75	0.69	-0.06							
5-yr	0.89	0.80	009							
10-yr	1.23	1.08	-0.15							
25-yr	1.47	1.28	-0.19							
100-yr	1.89	1.62	-0.27							

Basin 2:

The peak runoff will slightly increase in basin 2 due to the increase in impervious area. However, detention is proposed to reduce the peak runoff to pre developed conditions.

(I cur now m erb)							
	De (De						
Pre-Dev.	Post-Dev	Difference					
1.59	1.57	-0.02					
1.88	1.77	-0.11					
2.55	2.09	-0.46					
3.03	2.36	-0.67					
3.85	3.67	-0.18					
	Pre-Dev. 1.59 1.88 2.55 3.03	Pre-Dev. Post-Dev 1.59 1.57 1.88 1.77 2.55 2.09 3.03 2.36					

(Peak flow in cfs)

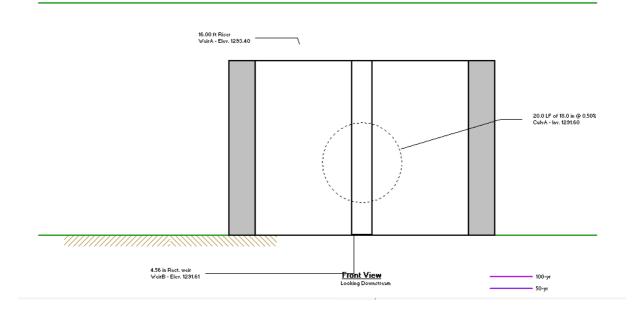
DETENTION BASIN 1:

The 100 year WSEL in the pond is 1293.5 and the berm is at elevation 1294.5. This provides 12 inches of freeboard. The release structure consists of a 4'x4' riser with a 4.5" weir and an 18" corrugated metal pipe outlet. See the grading plan for details.

Design Storm	Storage Cu. Ft	Elevation
2-yr	4,895	1287.41
10-yr	8,986	1287.84
25-yr	11,483	1288.04
50-yr	13,885	1288.15
100-yr	15,743	1288.23

1

Top of pond Elev. 1294.00



EROSION AND SEDIMENT CONTROL:

Erosion and sediment control will be achieved through the use of silt fences and rip rap ditch checks.

CONCLUSION:

The improvements to the site consist of adding parking, drives and a building. The peak runoff will increase due to the improvements replacing pervious area with impervious area. However, detention is proposed to lower the peak runoff to pre developed conditions.

CERTIFICATION:

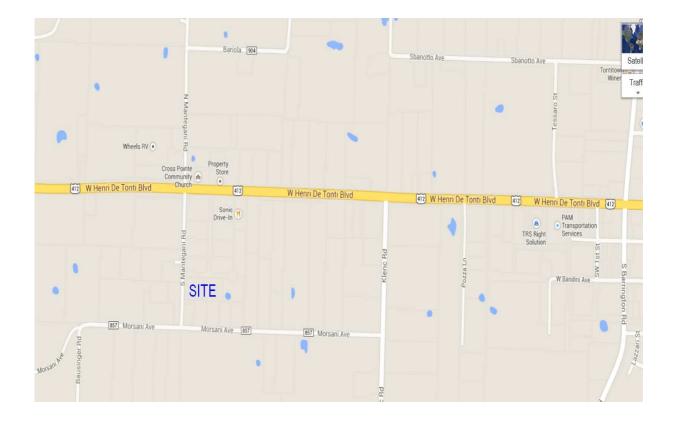
I, Geoffrey H. Bates, Registered Professional Engineer No. 9810 in the State of Arkansas, hereby certify that the drainage studies, reports, calculations, designs, and specifications contained in this report have been prepared in accordance with the requirements of the City of Tontitown. Further, I hereby acknowledge that the review of the drainage studies, reports, calculations, designs, and specifications by the City of Tontitown or its representatives cannot and does not relieve me from any professional responsibility or liability."

Sincerely,

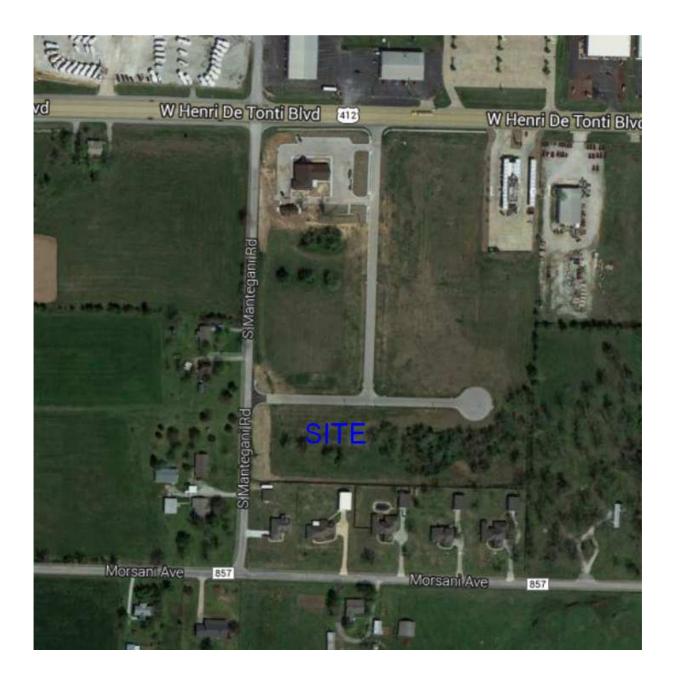
Bates & Associates, Inc.

Deoffrey Bates

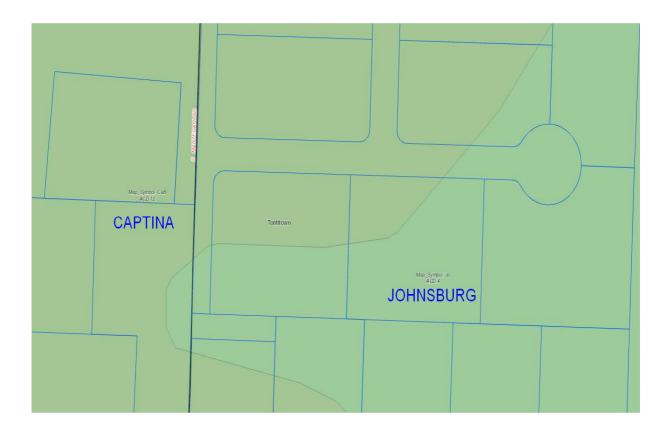
Geoffrey H. Bates, P.E. President of Engineering



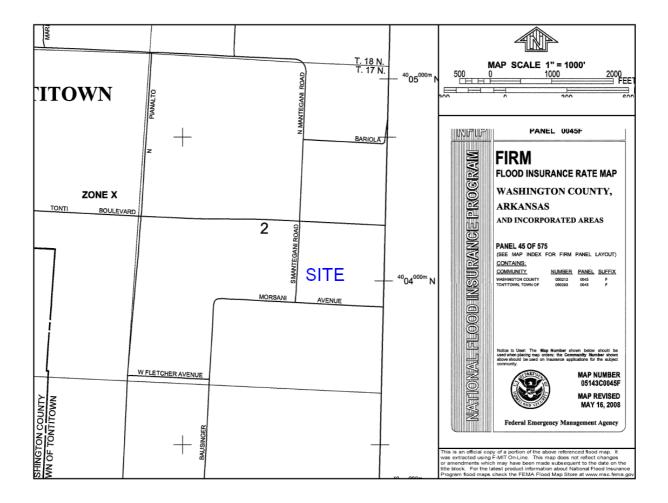
VICINITY MAP



AERIAL PHOTOGRAPH

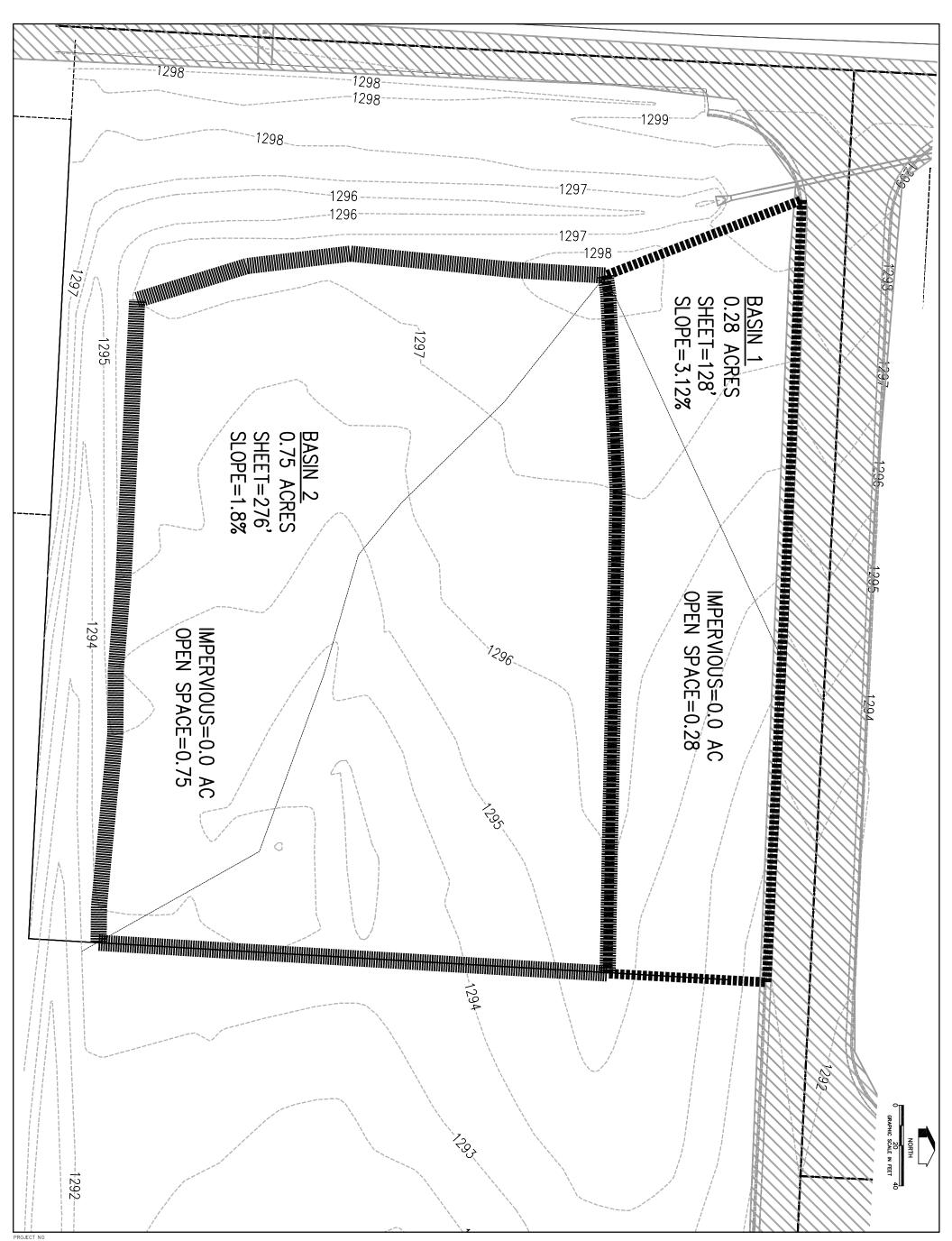


SOILS MAP



FEMA FIRM PANEL

RUNOFF CALCULATIONS



15-198

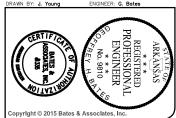
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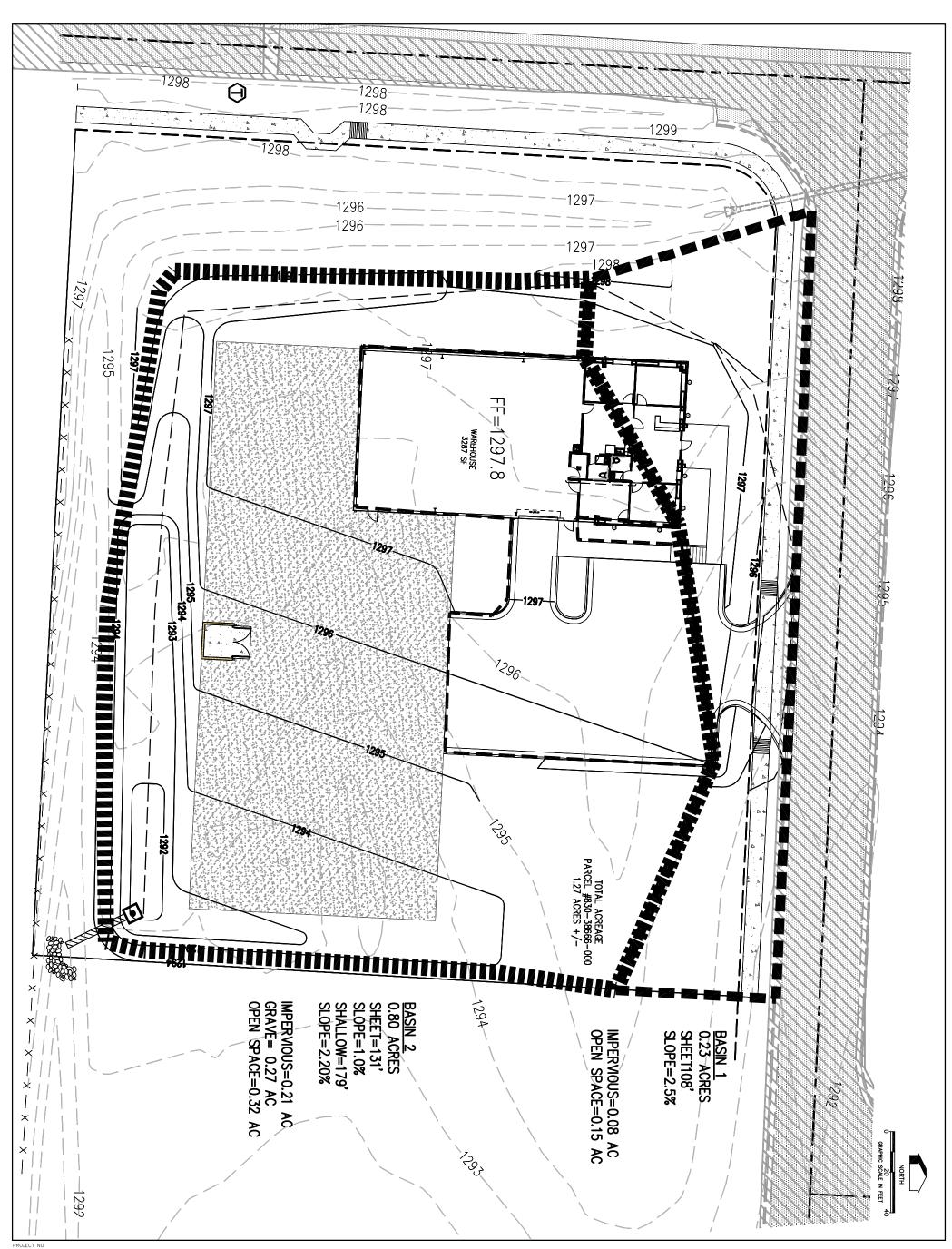
CENTRAL ELECTRICAL CONTRACTORS LARGE SCALE DEVELOPMENT PLANS

PRE DEVELOPED DRAINAGE MAP

TONTITOWN, ARKANSAS

REVISIONS	DATE
FIRST SUBMITTAL	07/07/15





15–198

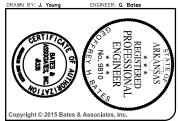
N Colt Square Dr FayettevIlle, Arkansas 72703 - 479.442.9350 - Fax 479.521.9350	
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CENTRAL ELECTRICAL CONTRACTORS LARGE SCALE DEVELOPMENT PLANS

POST DEVELOPED DRAINAGE MAP

TONTITOWN, ARKANSAS

REVISIONS	DATE
FIRST SUBMITTAL	07/07/15



Hydraflow Hydrographs by Intelisolve v9.2

Hydrograph Return Period Recap

Hydraflow Hydrographs by Intelisolve v9.2

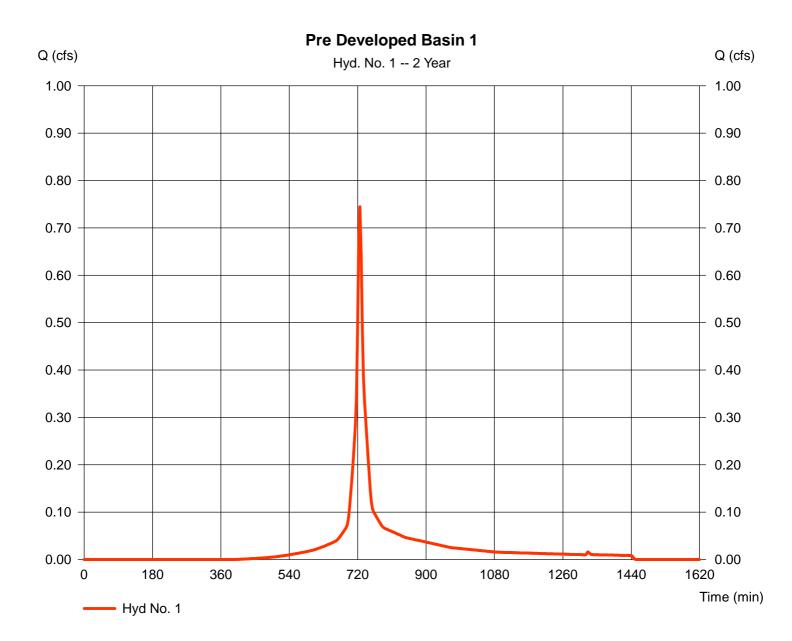
lyd. Io.	Hydrograph	Inflow				Hydrograph					
ю.	type (origin)	Hyd(s)	1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	description
1	SCS Runoff			0.745		0.888	1.228	1.470	1.710	1.890	Pre Developed Basin 1
2	SCS Runoff			1.593		1.878	2.551	3.026	3.499	3.852	Pre Developed Basin 2
3	SCS Runoff			0.685		0.803	1.081	1.276	1.471	1.617	Post Developed Basin 1
4	SCS Runoff			2.026		2.367	3.171	3.739	4.303	4.725	Post Developed Basin 2
5	Reservoir	4		1.569		1.766	2.094	2.359	3.128	3.670	Detention Pond

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip. Storm duration	 SCS Runoff 2 yrs 3 min 0.280 ac 0.0 % TR55 4.08 in 24 hrs 	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution Shape factor	 = 0.745 cfs = 726 min = 2,495 cuft = 86 = 0 ft = 8.80 min = Type III = 484
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 1

Pre Developed Basin 1

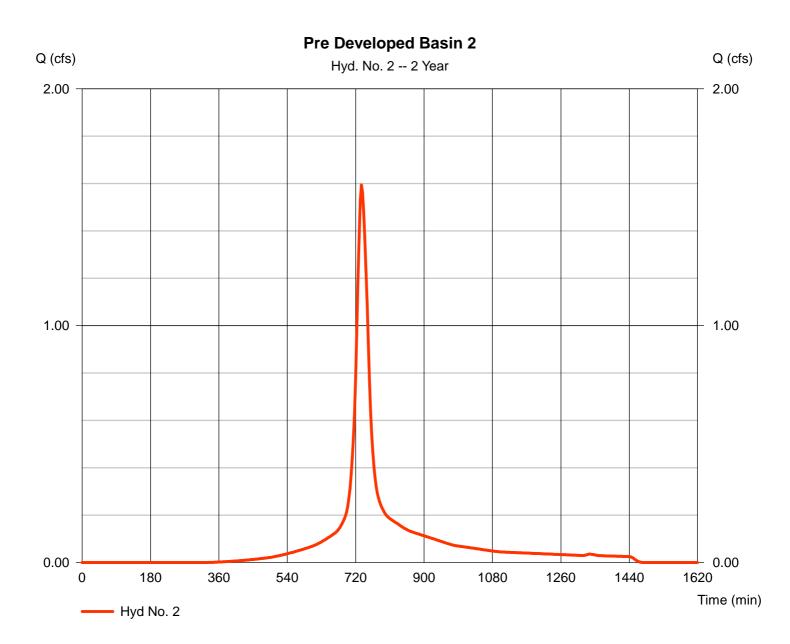
Description	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 128.0 = 4.08 = 3.12		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.85	+	0.00	+	0.00	=	8.85
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved = 0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	$\begin{array}{l} = \ 0.00 \\ = \ 0.00 \\ = \ 0.00 \\ = \ 0.015 \\ = \ 0.00 \\ = \ 0.0 \end{array}$		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

Pre Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 1.593 cfs
Storm frequency	= 2 yrs	Time to peak	= 735 min
Time interval	= 3 min	Hyd. volume	= 7,693 cuft
Drainage area	= 0.750 ac	Curve number	= 89
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 20.40 min
Total precip.	= 4.08 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 2

Pre Developed Basin 2

Description	<u>A</u>		<u>B</u>		<u>C</u>		Totals
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 276.0 = 4.08 = 1.80		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 20.39	+	0.00	+	0.00	=	20.39
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved = 0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							20.40 min

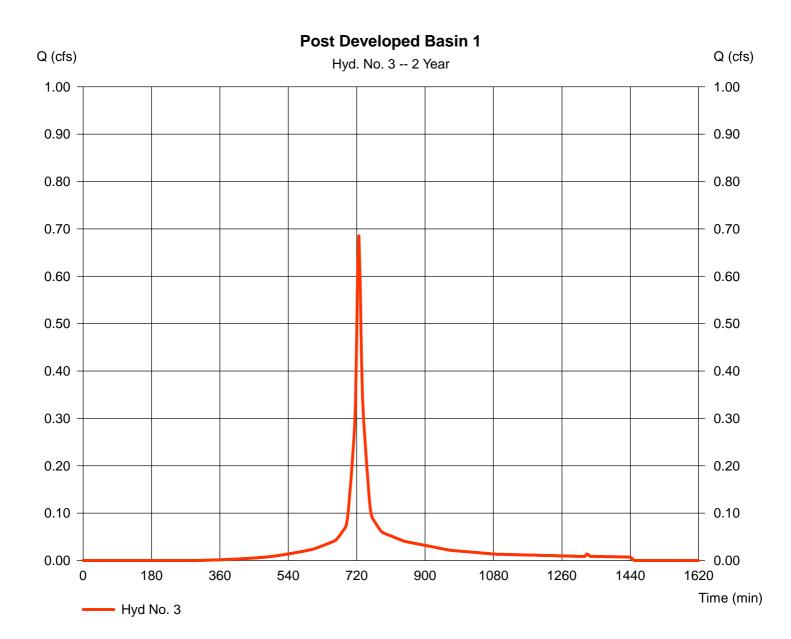
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

Post Developed Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.685 cfs
Storm frequency	= 2 yrs	Time to peak	= 726 min
Time interval	= 3 min	Hyd. volume	= 2,344 cuft
Drainage area	= 0.230 ac	Curve number	= 90*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.40 min
Total precip.	= 4.08 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.080 x 98) + (0.150 x 86)] / 0.230



Hyd. No. 3

Post Developed Basin 1

Description	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 108.0 = 4.08 = 2.50		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.44	+	0.00	+	0.00	=	8.44
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved = 0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	$\begin{array}{l} = & 0.00 \\ = & 0.00 \\ = & 0.00 \\ = & 0.015 \\ = & 0.00 \\ = & 0.0 \end{array}$		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.40 min

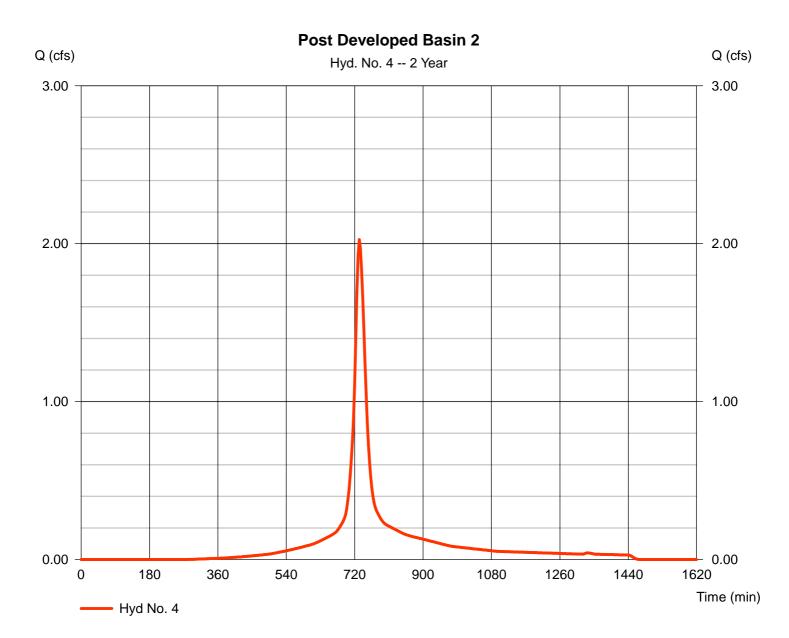
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

Post Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 2.026 cfs
Storm frequency	= 2 yrs	Time to peak	= 732 min
Time interval	= 3 min	Hyd. volume	= 9,266 cuft
Drainage area	= 0.800 ac	Curve number	= 91*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 15.50 min
Total precip.	= 4.08 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



Hyd. No. 4

Post Developed Basin 2

Description		<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.150 131.0 4.08 1.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	=	14.21	+	0.00	+	0.00	=	14.21
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=	179.00 2.20 Unpaved 2.39	Ь	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	=	1.25	+	0.00	+	0.00	=	1.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= = =	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.50 min	

Hydraflow Hydrographs by Intelisolve v9.2

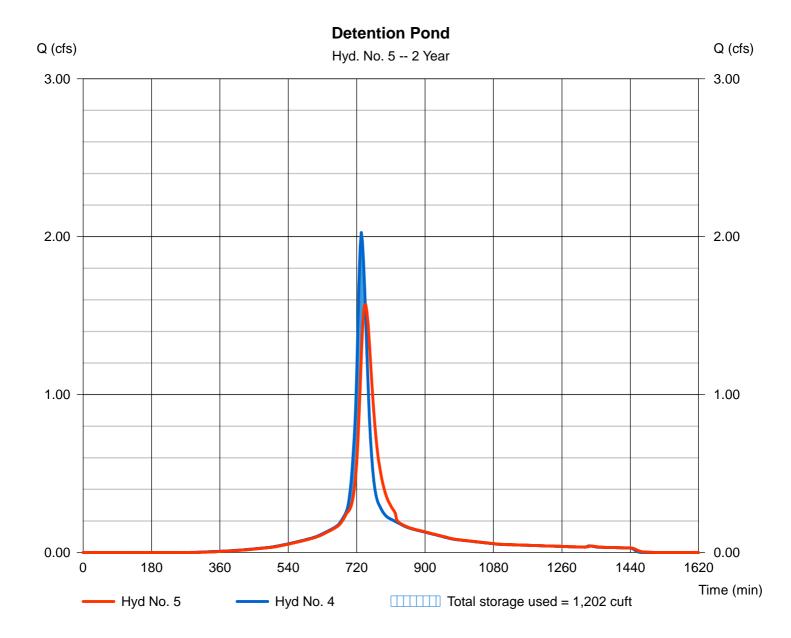
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Detention Pond

Hydrograph type	= Reservoir	Peak discharge	= 1.569 cfs
Storm frequency	= 2 yrs	Time to peak	= 744 min
Time interval	= 3 min	Hyd. volume	= 9,264 cuft
Inflow hyd. No.	= 4 - Post Developed Basin 2	Max. Elevation	= 1292.93 ft
Reservoir name	= <new pond=""></new>	Max. Storage	= 1,202 cuft
Reservoir name	= <new pond=""></new>	Max. Storage	= 1,202 cuft

Storage Indication method used.



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Pond Report

Hydraflow Hydrographs by Intelisolve v9.2

Pond No. 1 - <New Pond>

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 1291.60 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	1291.60	10	0	0
0.40	1292.00	412	65	65
1.40	1293.00	2,308	1,232	1,296
2.40	1294.00	6,245	4,116	5,412

Culvert / Orifice Structures

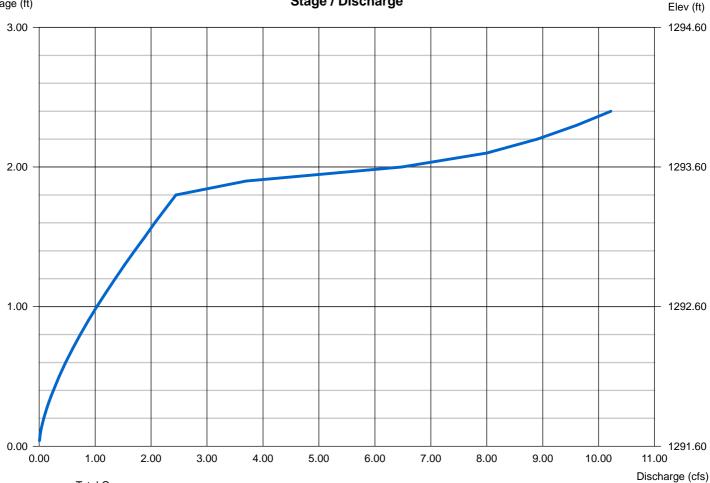
	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 18.00	0.00	0.00	0.00	Crest Len (ft)	= 16.00	0.38	0.00	0.00
Span (in)	= 18.00	0.00	0.00	0.00	Crest El. (ft)	= 1293.40	1291.61	0.00	0.00
No. Barrels	= 1	0	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 1291.60	0.00	0.00	0.00	Weir Type	= Riser	Rect		
Length (ft)	= 20.00	0.00	0.00	0.00	Multi-Stage	= Yes	Yes	No	No
Slope (%)	= 0.50	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Weir Structures

Stage (ft)

Stage / Discharge



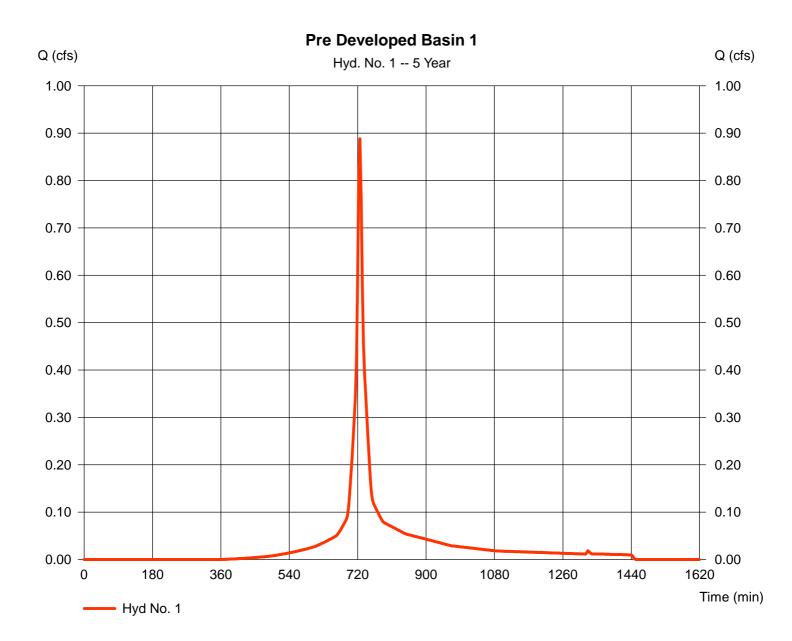
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Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip. Storm duration	 SCS Runoff 5 yrs 3 min 0.280 ac 0.0 % TR55 4.65 in 24 hrs 	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution Shape factor	 = 0.888 cfs = 726 min = 2,994 cuft = 86 = 0 ft = 8.80 min = Type III = 484
Storm duration	= 24 hrs	Shape factor	= 484

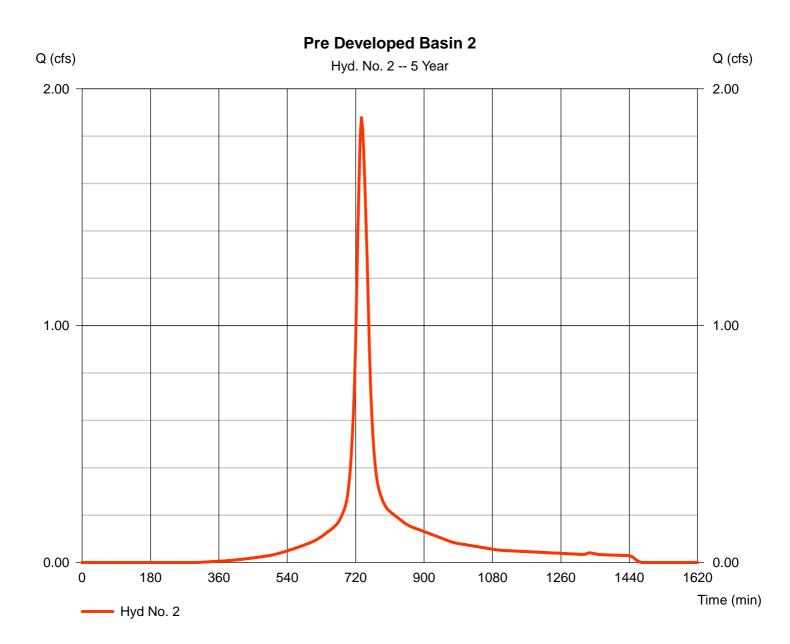


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

Pre Developed Basin 2

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip.	= 3 min = 0.750 ac = 0.0 % = TR55	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution	
Total precip. Storm duration	= 1R55 = 4.65 in = 24 hrs	Distribution Shape factor	= 20.40 min = Type III = 484



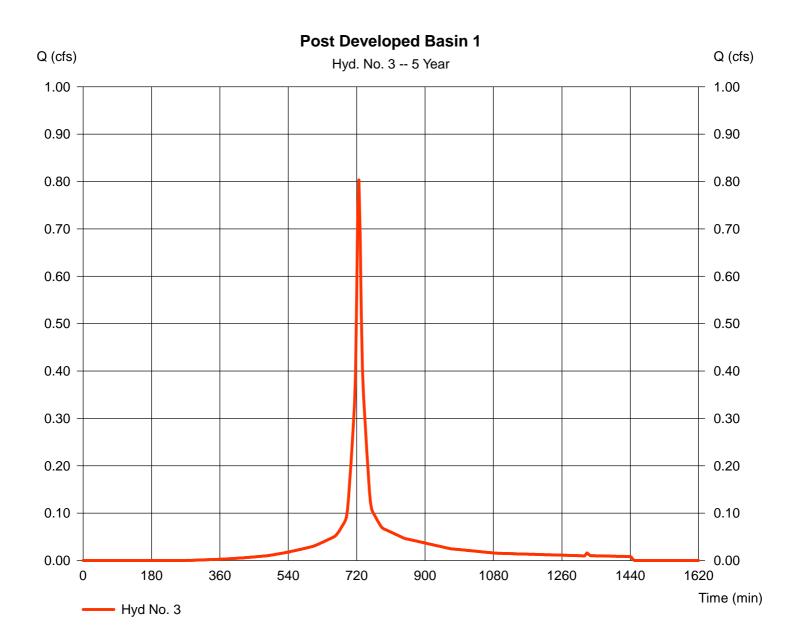
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

Post Developed Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.803 cfs
Storm frequency	= 5 yrs	Time to peak	= 726 min
Time interval	= 3 min	Hyd. volume	= 2,770 cuft
Drainage area	= 0.230 ac	Curve number	= 90*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.40 min
Total precip.	= 4.65 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.080 x 98) + (0.150 x 86)] / 0.230



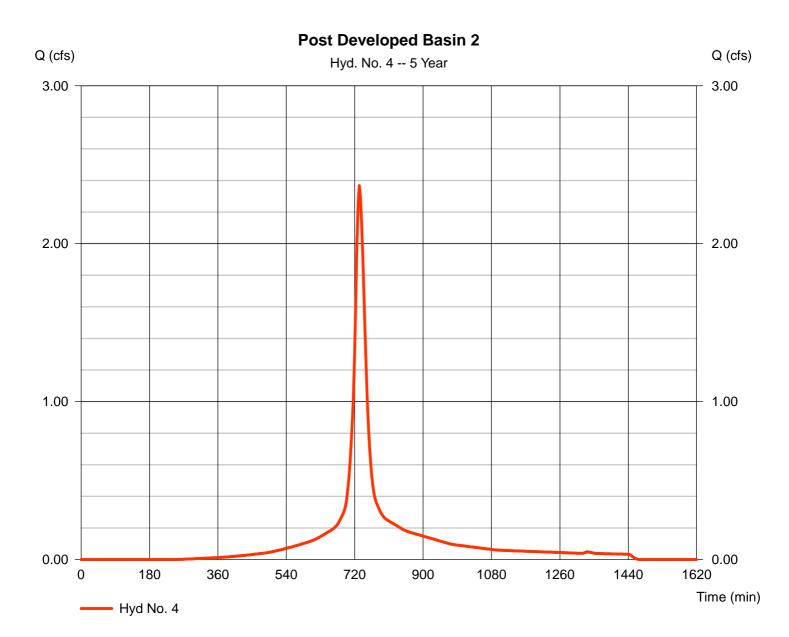
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

Post Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 2.367 cfs
Storm frequency	= 5 yrs	Time to peak	= 732 min
Time interval	= 3 min	Hyd. volume	= 10,910 cuft
Drainage area	= 0.800 ac	Curve number	= 91*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 15.50 min
Total precip.	= 4.65 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484
Basin Šlope Tc method Total precip.	= 0.0 % = TR55 = 4.65 in	Hydraulic length Time of conc. (Tc) Distribution	= 0 ft = 15.50 min = Type III

* Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



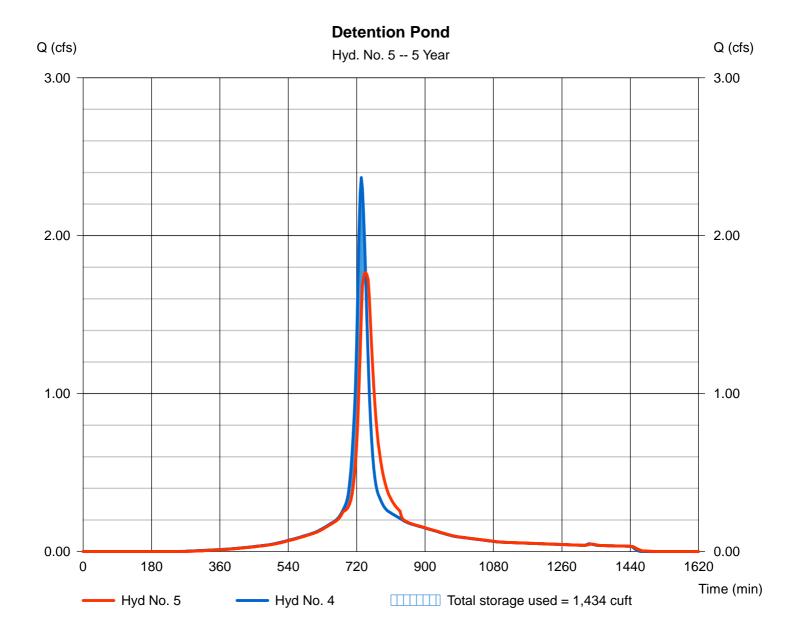
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Detention Pond

Hydrograph type =	Reservoir	Peak discharge	= 1.766 cfs
Storm frequency =	= 5 yrs	Time to peak	= 744 min
Time interval =	= 3 min	Hyd. volume	= 10,908 cuft
Inflow hyd. No. =	4 - Post Developed Basin 2	Max. Elevation	= 1293.04 ft
Reservoir name =	= <new pond=""></new>	Max. Storage	= 1,434 cuft

Storage Indication method used.

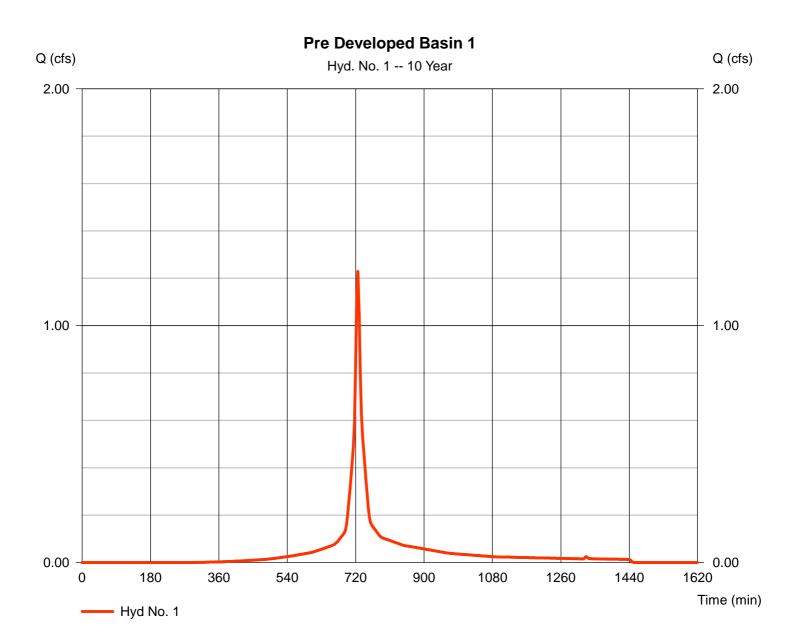


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip.	= 3 min = 0.280 ac = 0.0 % = TR55 = 6.00 in	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

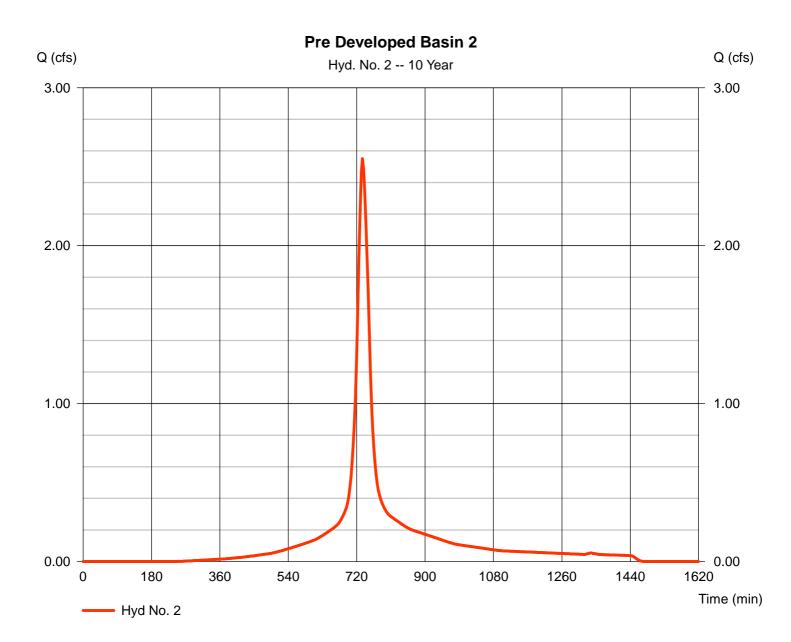


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

Pre Developed Basin 2

Hydrograph type Storm frequency		Peak discharge Time to peak	= 2.551 cfs = 735 min
Time interval	= 3 min	Hyd. volume	= 12,570 cuft
Drainage area	= 0.750 ac	Curve number	= 89
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 20.40 min
Total precip.	= 6.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



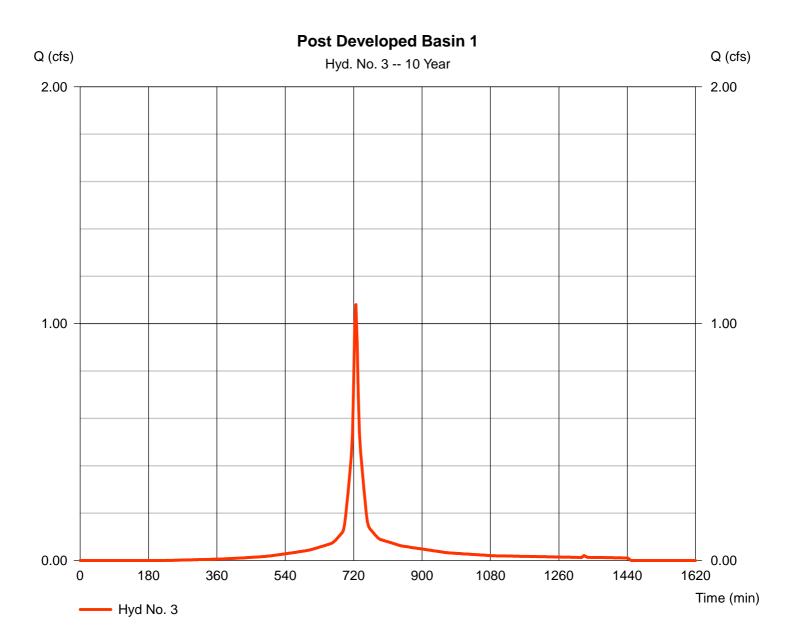
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

Post Developed Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.081 cfs
Storm frequency	= 10 yrs	Time to peak	= 726 min
Time interval	= 3 min	Hyd. volume	= 3,793 cuft
Drainage area	= 0.230 ac	Curve number	= 90*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.40 min
Total precip.	= 6.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.080 x 98) + (0.150 x 86)] / 0.230



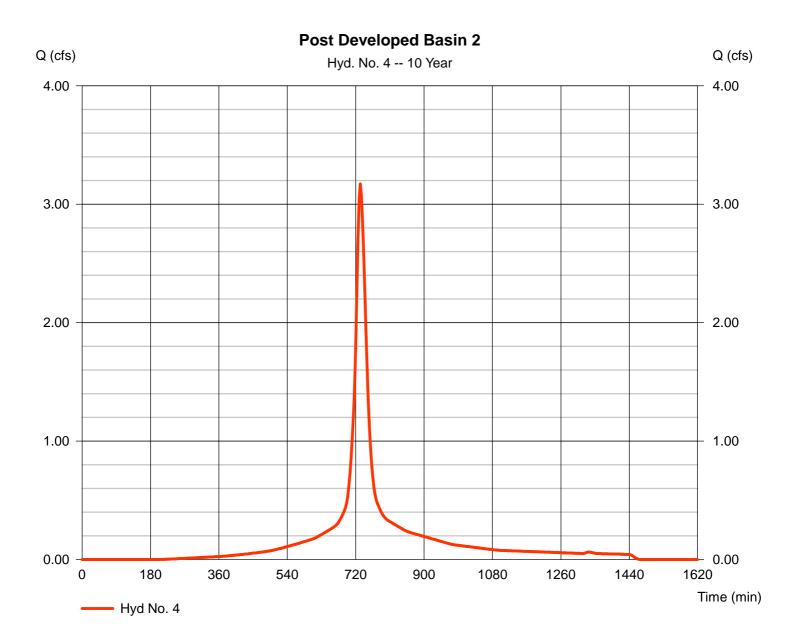
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

Post Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 3.171 cfs
Storm frequency	= 10 yrs	Time to peak	= 732 min
Time interval	= 3 min	Hyd. volume	= 14,846 cuft
Drainage area	= 0.800 ac	Curve number	= 91*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 15.50 min
Total precip.	= 6.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



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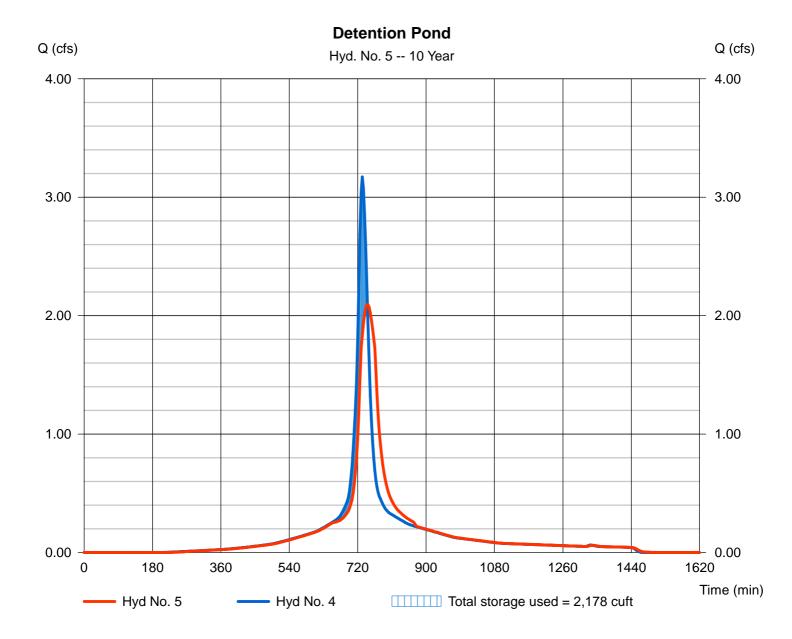
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Detention Pond

Reservoir	Peak discharge	= 2.094 cfs
0 yrs	Time to peak	= 747 min
5 min	Hyd. volume	= 14,844 cuft
 Post Developed Basin 2 	Max. Elevation	= 1293.22 ft
New Pond>	Max. Storage	= 2,178 cuft
	0 yrs min - Post Developed Basin 2	0 yrs Time to peak min Hyd. volume - Post Developed Basin 2 Max. Elevation

Storage Indication method used.

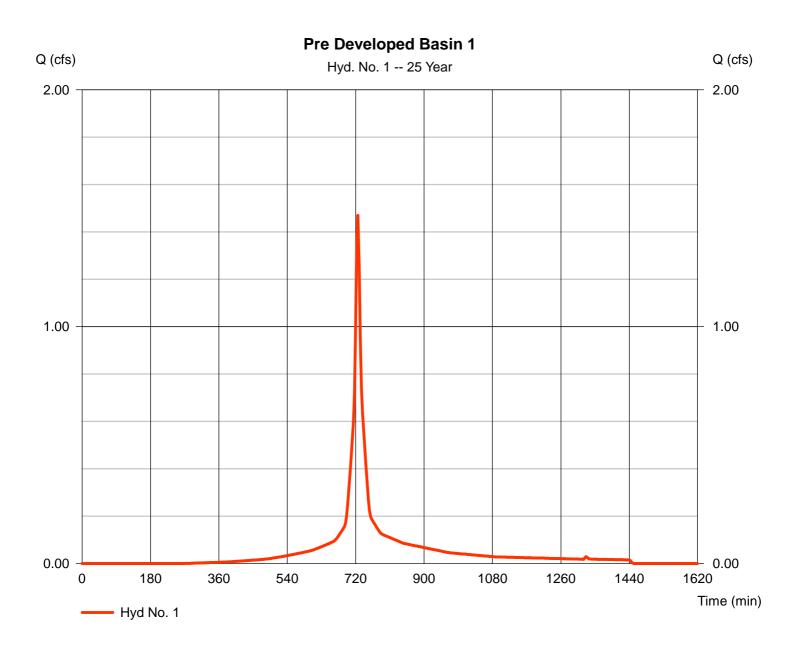


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type Storm frequency		Peak discharge Time to peak	= 1.470 cfs = 726 min
Time interval	= 3 min	Hyd. volume	= 5,076 cuft
Drainage area	= 0.280 ac	Curve number	= 86
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.80 min
Total precip.	= 6.96 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

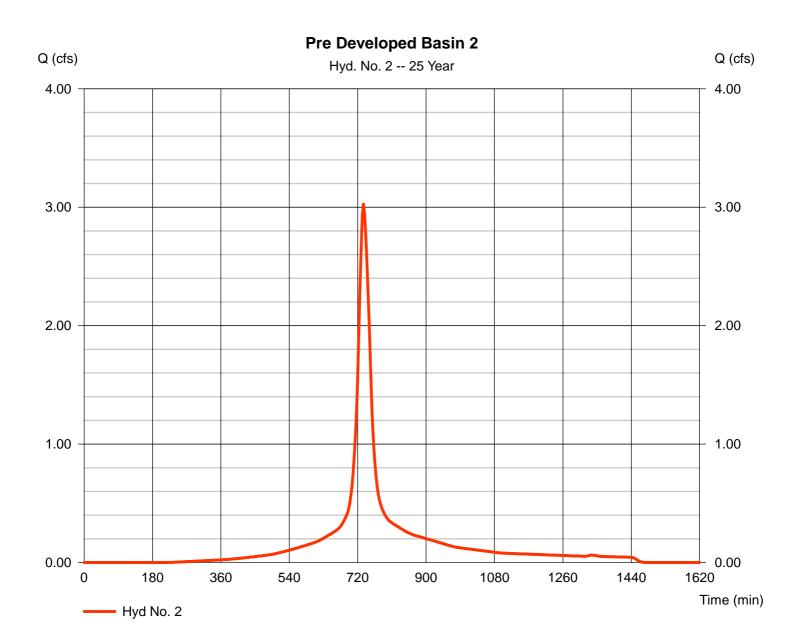


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

Pre Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 3.026 cfs
Storm frequency	= 25 yrs	Time to peak	= 735 min
Time interval	= 3 min	Hyd. volume	= 15,048 cuft
Drainage area	= 0.750 ac	Curve number	= 89
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 20.40 min
Total precip.	= 6.96 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



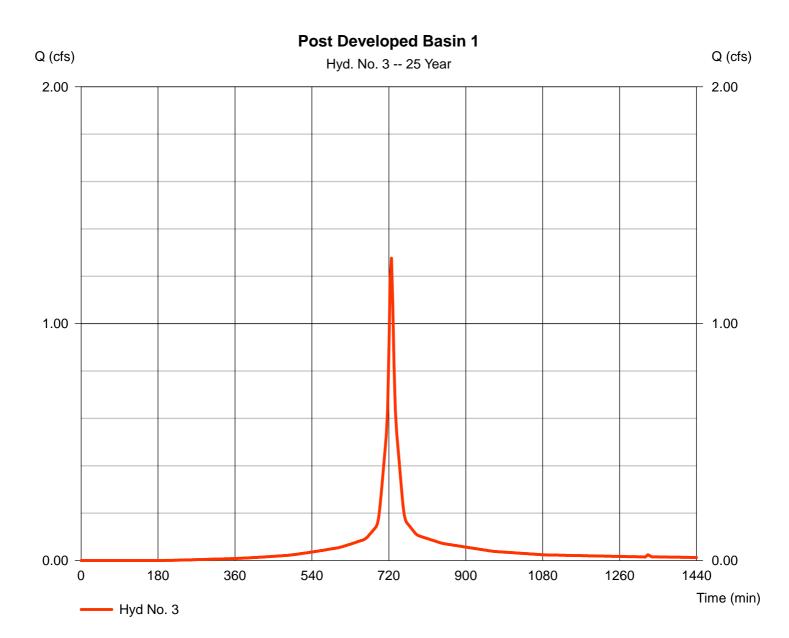
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

Post Developed Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.276 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 3 min	Hyd. volume	= 4,527 cuft
Drainage area	= 0.230 ac	Curve number	= 90*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.40 min
Total precip.	= 6.96 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.080 x 98) + (0.150 x 86)] / 0.230



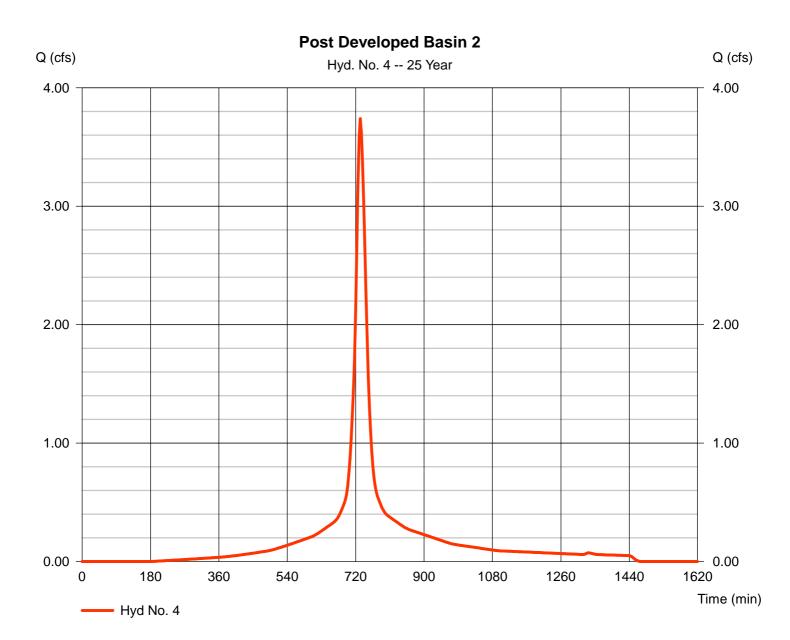
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

Post Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 3.739 cfs
Storm frequency	= 25 yrs	Time to peak	= 732 min
Time interval	= 3 min	Hyd. volume	= 17,667 cuft
Drainage area	= 0.800 ac	Curve number	= 91*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 15.50 min
Total precip.	= 6.96 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



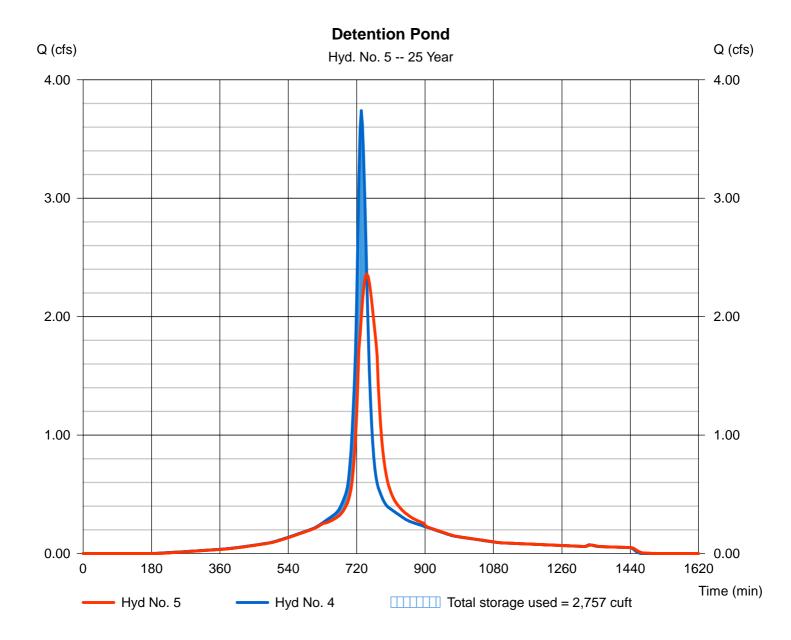
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Detention Pond

Hydrograph type	= Reservoir	Peak discharge	= 2.359 cfs
Storm frequency	= 25 yrs	Time to peak	= 747 min
Time interval	= 3 min	Hyd. volume	= 17,666 cuft
Inflow hyd. No.	= 4 - Post Developed Basin 2	Max. Elevation	= 1293.36 ft
Reservoir name	= <new pond=""></new>	Max. Storage	= 2,757 cuft

Storage Indication method used.

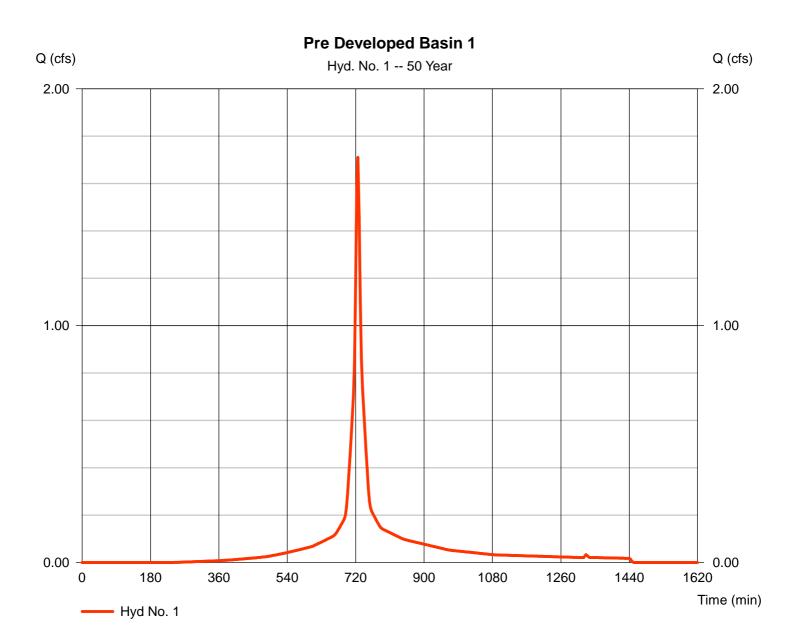


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip. Storm duration	= 3 min = 0.280 ac = 0.0 % = TR55 = 7.92 in	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution Shape factor	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

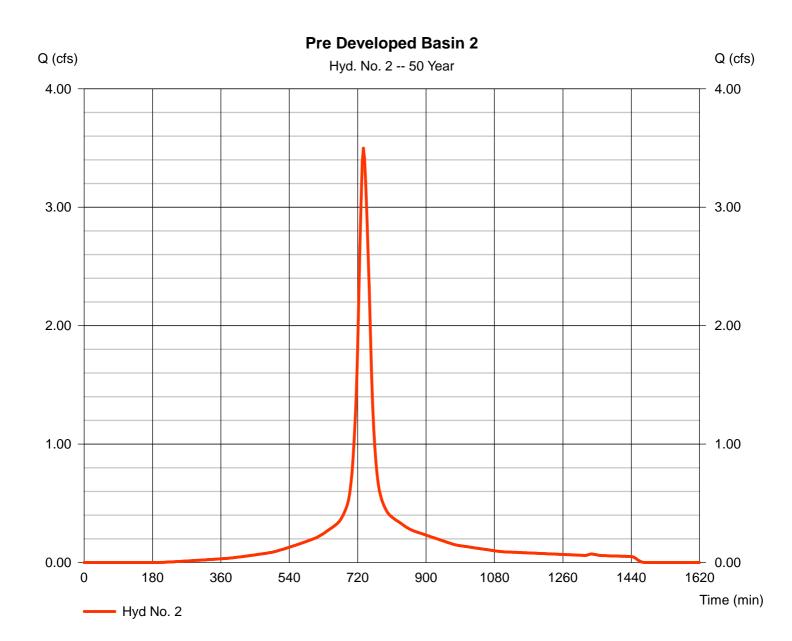


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

Pre Developed Basin 2

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip. Storm duration	= 3 min = 0.750 ac = 0.0 % = TR55 = 7.92 in	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution Shape factor	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



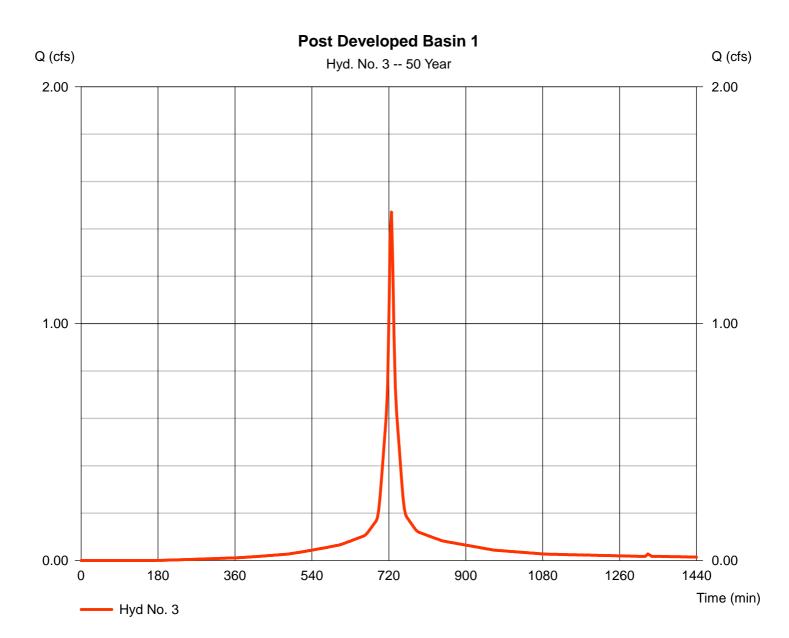
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

Post Developed Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.471 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 3 min	Hyd. volume	= 5,265 cuft
Drainage area	= 0.230 ac	Curve number	= 90*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.40 min
Total precip.	= 7.92 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.080 x 98) + (0.150 x 86)] / 0.230



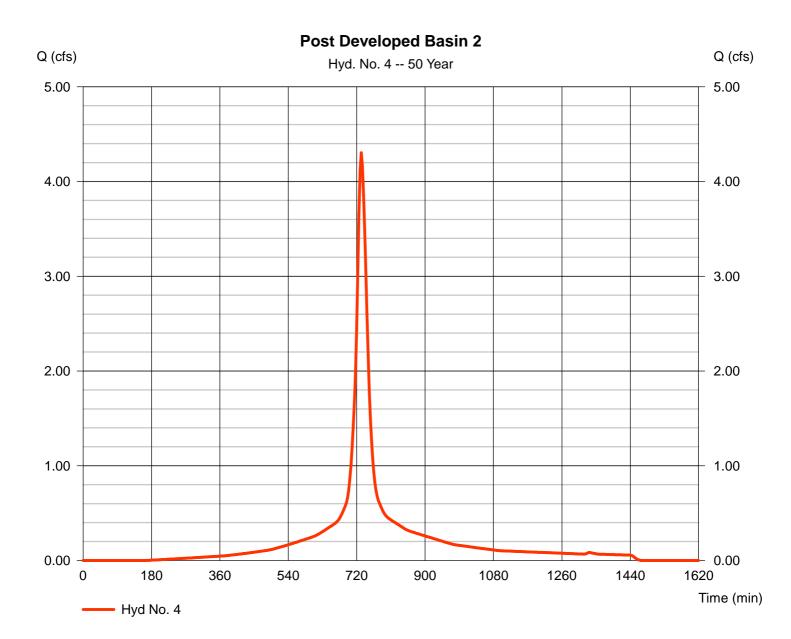
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

Post Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 4.303 cfs
Storm frequency	= 50 yrs	Time to peak	= 732 min
Time interval	= 3 min	Hyd. volume	= 20,500 cuft
Drainage area	= 0.800 ac	Curve number	= 91*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 15.50 min
Total precip.	= 7.92 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



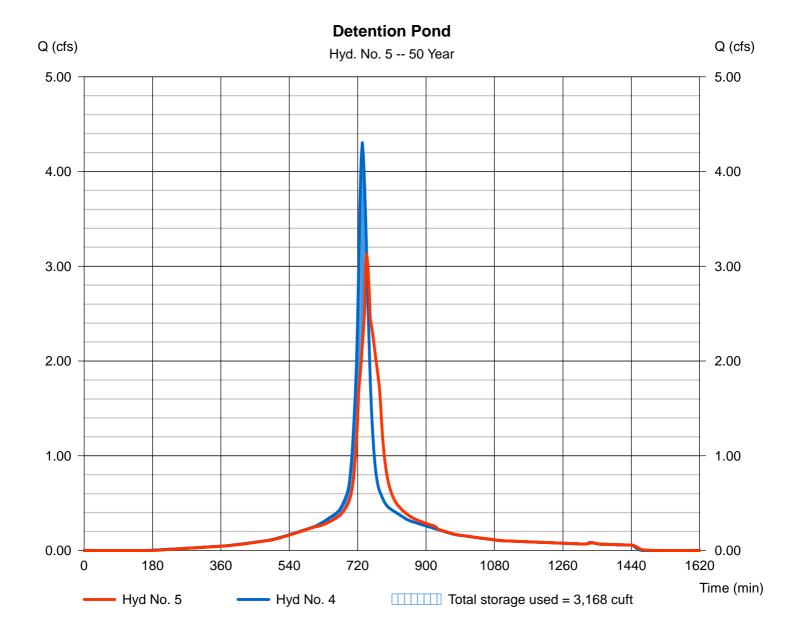
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Detention Pond

Hydrograph type	= Reservoir	Peak discharge	= 3.128 cfs
Storm frequency	= 50 yrs	Time to peak	= 744 min
Time interval	= 3 min	Hyd. volume	= 20,499 cuft
Inflow hyd. No.	= 4 - Post Developed Basin 2	Max. Elevation	= 1293.46 ft
Reservoir name	= <new pond=""></new>	Max. Storage	= 3,168 cuft

Storage Indication method used.

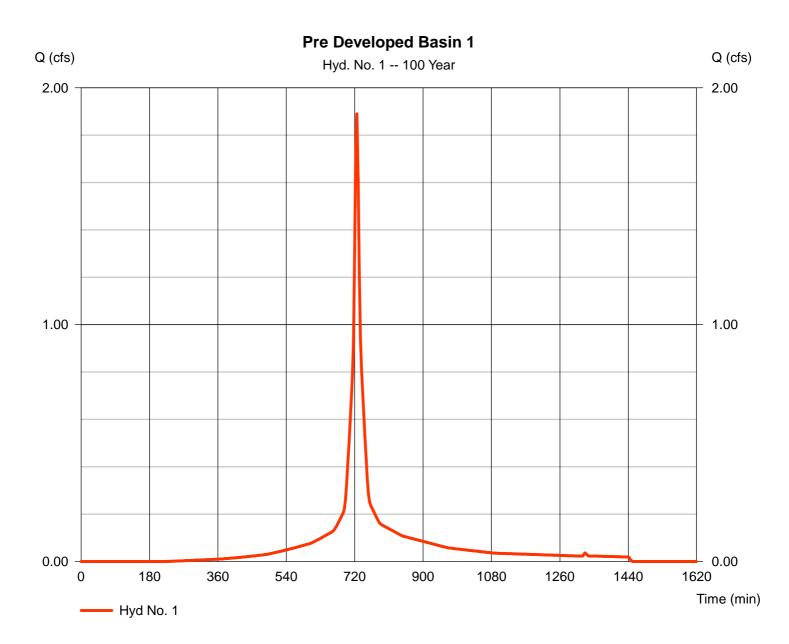


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

Pre Developed Basin 1

Hydrograph type Storm frequency Time interval Drainage area Basin Slope Tc method Total precip. Storm duration	= 100 yrs = 3 min = 0.280 ac = 0.0 % = TR55 = 8.64 in	Peak discharge Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution Shape factor	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

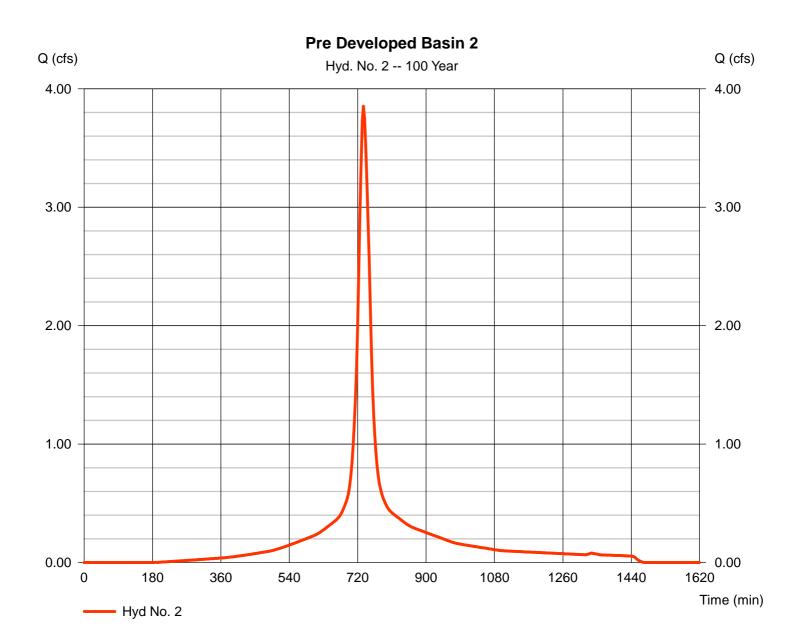


Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

Pre Developed Basin 2

Drainage area= 0.750 acCurve number= 89Basin Slope= 0.0 %Hydraulic length= 0 ft	Hydrograph type Storm frequency Time interval	quency = 100 yrs	Peak discharge Time to peak Hyd. volume	= 3.852 cfs = 735 min = 19,419 cuft
Tc method= TR55Time of conc. (Tc)= 20.40 minTotal precip.= 8.64 inDistribution= Type IIIStorm duration= 24 hrsShape factor= 484	Drainage area	area = 0.750 ac	Curve number	= 89
	Basin Slope	be = 0.0 %	Hydraulic length	= 0 ft
	Tc method	d = TR55	Time of conc. (Tc)	= 20.40 min
	Total precip.	ip. = 8.64 in	Distribution	= Type III



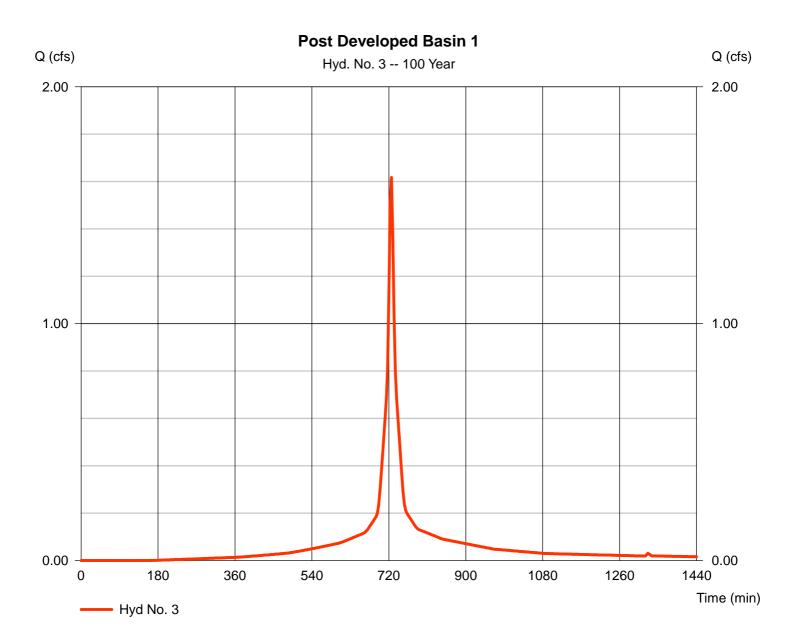
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

Post Developed Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.617 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 3 min	Hyd. volume	= 5,820 cuft
Drainage area	= 0.230 ac	Curve number	= 90*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 8.40 min
Total precip.	= 8.64 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.080 x 98) + (0.150 x 86)] / 0.230



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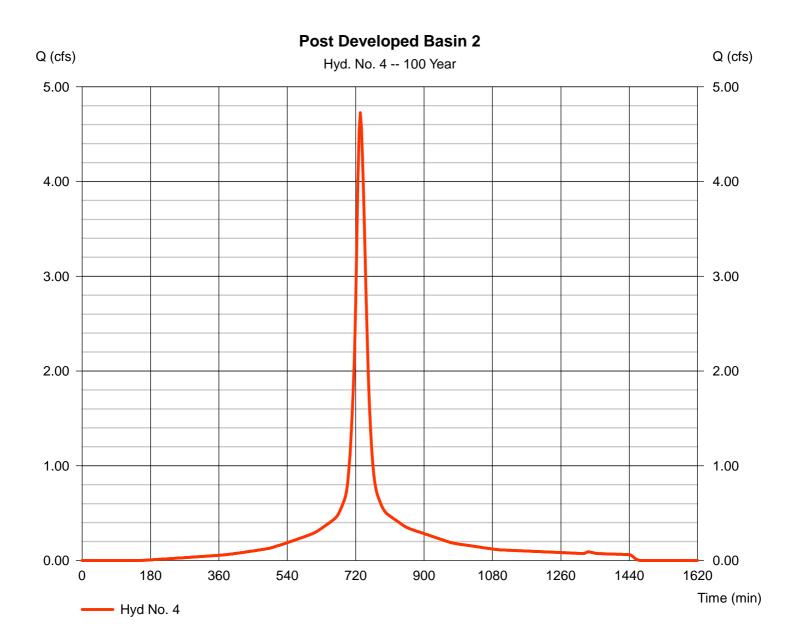
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

Post Developed Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 4.725 cfs
Storm frequency	= 100 yrs	Time to peak	= 732 min
Time interval	= 3 min	Hyd. volume	= 22,631 cuft
Drainage area	= 0.800 ac	Curve number	= 91*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 15.50 min
Total precip.	= 8.64 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(0.320 x 89) + (0.270 x 89) + (0.210 x 98)] / 0.800



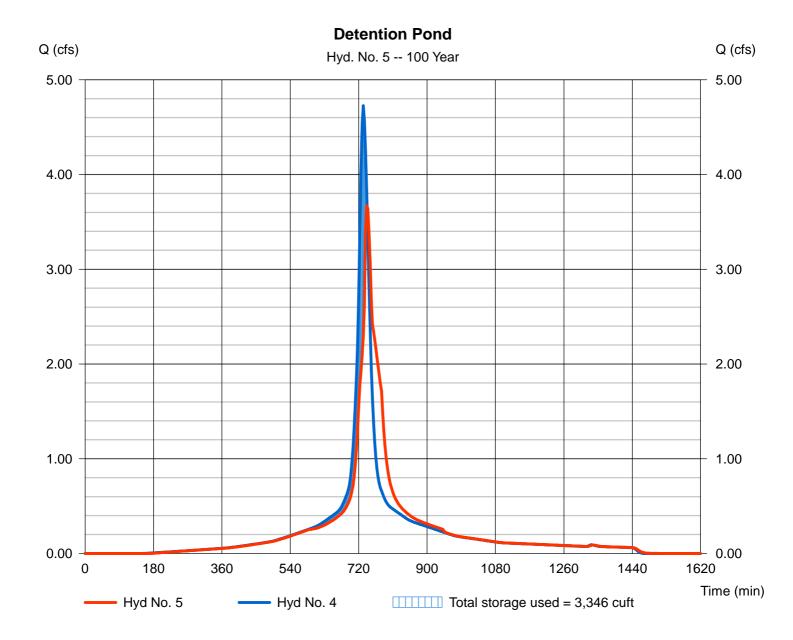
Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Detention Pond

Hydrograph type	= Reservoir	Peak discharge	= 3.670 cfs
Storm frequency	= 100 yrs	Time to peak	= 741 min
Time interval	= 3 min	Hyd. volume	= 22,630 cuft
Inflow hyd. No.	= 4 - Post Developed Basin 2	Max. Elevation	= 1293.50 ft
Reservoir name	= <new pond=""></new>	Max. Storage	= 3,346 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs by Intelisolve v9.2

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	63.3915	13.3000	0.8386						
3	0.0000	0.0000	0.0000						
5	64.9066	13.0000	0.7920						
10	79.4587	14.3000	0.8048						
25	104.1785	15.8000	0.8275						
50	102.8257	15.1000	0.7991						
100	95.5744	13.9000	0.7608						

File name: Tontitown.IDF

Intensity = B / (Tc + D)^E

Return Period		Intensity Values (in/hr)										
(Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.54	4.52	3.84	3.35	2.98	2.69	2.45	2.26	2.10	1.96	1.84	1.73
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.58	5.42	4.64	4.07	3.64	3.30	3.02	2.80	2.60	2.44	2.30	2.17
10	7.34	6.10	5.24	4.62	4.14	3.76	3.45	3.19	2.97	2.79	2.62	2.48
25	8.45	7.07	6.11	5.39	4.84	4.40	4.04	3.74	3.48	3.26	3.07	2.90
50	9.35	7.83	6.77	5.99	5.38	4.90	4.51	4.18	3.90	3.65	3.44	3.26
100	10.21	8.54	7.39	6.55	5.90	5.38	4.96	4.60	4.30	4.04	3.82	3.62

Tc = time in minutes. Values may exceed 60.

Precip. file name: Sprindale.pcp								
	Rainfall Precipitation Table (in)							
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	0.00	4.08	0.00	4.65	6.00	6.96	7.92	8.64
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

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